# TRAFFIC FORECAST TECHNICAL MEMORANDUM 

KINSTON BYPASS ALTERNATIVES STUDY<br>TIP Project R-2553<br>Lenoir, jones \& CRAVEN COUNTIES

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## TABLE OF CONTENTS

1.0 INTRODUCTION ..... 1
1.1 Project Location ..... 1
1.2 Roadway Network ..... 1
2.0 PROJECT DESCRIPTION ..... 4
2.1 History. ..... 4
2.2 Fiscal Constraint ..... 5
2.3 Alternatives ..... 5
3.0 DATA COLLECTION ..... 10
3.1 Classification Counts ..... 10
3.2 Turn Movement Counts ..... 10
3.3 Convert Class Counts to AADT ..... 10
3.4 Convert TMC to Quadrant Volumes ..... 10
3.5 Comparison with Historic AADTs ..... 11
4.0 TRAVEL DEMAND MODEL ..... 13
4.1 Model Review ..... 13
4.2 Base Year and No Build Network Modifications ..... 15
4.3 Development of Build Networks ..... 16
5.0 DEVELOPMENT OF PROJECT LEVEL TRAFFIC FORECASTS ..... 18
5.1 Development of Mainline Volumes ..... 18
5.2 Development of Turn Movements ..... 18
5.3 No Build Alternative ..... 19
5.4 Build Alternatives ..... 19
5.5 Screenline Comparison of Alternative Forecasts ..... 20
6.0 TRAFFIC FACTORS ..... 23
6.1 Directional Splits (D) ..... 23
6.2 Design Hour Volume Percentage (K) ..... 23
6.3 Truck Percentages ..... 24
7.0 TRAFFIC FORECASTS ..... 26
7.1 No Build Alternatives ..... 26
7.2 Build Alternatives ..... 26
8.0 TRAFFIC ESTIMATES FOR SHALLOW BYPASS ..... 27

## LIST OF TABLES

Table 1. Bypass Alternatives Evaluated ..... 5
Table 2. Proposed Interchanges, Length and Travel Time along Bypass Alternatives ..... 9
Table 3. Socio-Economic Data from Model ..... 14
Table 4. Population Estimates and Projections for Lenoir County ..... 14
Table 5: Total Employment for GTP TAZs in BY 2011 and FY 2040 ..... 15
Table 6. Total Trips in Base Year 2011 and No Build 2040 Models ..... 15
Table 7. Base Year and Future Year Network Modifications ..... 16
Table 8. Screenline Analysis of 2040 Forecasts ..... 22
Table 9. Traffic Design Factors - D, K and HV at Key Locations ..... 25
Table 10. Scenarios for AADT Estimates with Shallow Bypass ..... 27
Table 11. Screenline Summary for Shallow Bypass Estimates ..... 28
LIST OF FIGURES
Figure 1. Study Area Map ..... 3
Figure 2. Kinston Bypass Alternatives ..... 8
Figure 3. Historical AADT Map ..... 12
Figure 4. TransCAD Model Network with Bypass Alternatives ..... 17
Figure 5. Screenlines for Comparing Forecasts ..... 21

## LIST OF APPENDICES

APPENDIX A TRAFFIC FORECASTS
APPENDIX B FORECAST DOCUMENTATION TABLES
TRAFFIC ESTIMATES FOR SHALLOW BYPASS INTERCHANGE OPTIONS DETAILED TRAFFIC COUNTS (ELECTRONIC TRANSMITTAL ONLY) TRANSCAD MODEL FILES (ELECTRONIC TRANSMITTAL ONLY)

## APPENDIX A - Traffic Forecasts

A1 Scenario 1-2015 Base Year Existing Conditions
A2 Scenario 2 - 2015 Base Year Build Alternative 1 (Upgrade US 70)
A3 Scenario 3A - 2015 Base Year Build Alternative 1S-S5 (Shallow Bypass with 5 interchanges) Scenario 3B - 2015 Base Year Build Alternative 1S-P4 (Shallow Bypass with 4 interchanges \& rerouting of existing US 17 to NC 58)
Scenario 4-2015 Base Year Build Alternative 11
Scenario 5-2015 Base Year Build Alternative 12
Scenario 6-2015 Base Year Build Alternative 31
Scenario 7-2015 Base Year Build Alternative 32
Scenario 8-2015 Base Year Build Alternative 35
Scenario 9-2015 Base Year Build Alternative 36
Scenario 10-2015 Base Year Build Alternative 51
Scenario 11-2015 Base Year Build Alternative 52
Scenario 12-2015 Base Year Build Alternative 63
Scenario 13-2015 Base Year Build Alternative 65
Scenario 14-2040 Horizon Year No Build
Scenario 15-2040 Horizon Year Build Alternative 1 (Upgrade US 70)
Scenario 16A - 2040 Horizon Year Build Alternative 1S-S5 (Shallow Bypass with 5 interchanges)
Scenario 16B - 2040 Horizon Year Build Alternative 1S-P4 (Shallow Bypass with 4 interchanges \& rerouting of existing US 17 to NC 58)
Scenario 17 - 2040 Horizon Year Build Alternative 11
Scenario 18-2040 Horizon Year Build Alternative 12
Scenario 19-2040 Horizon Year Build Alternative 31
Scenario 20-2040 Horizon Year Build Alternative 32
Scenario 21 - 2040 Horizon Year Build Alternative 35
Scenario 22 - 2040 Horizon Year Build Alternative 36
Scenario 23-2040 Horizon Year Build Alternative 51
Scenario 24-2040 Horizon Year Build Alternative 52
Scenario 25-2040 Horizon Year Build Alternative 63
Scenario 26-2040 Horizon Year Build Alternative 65

## APPENDIX B - Forecast Documentation Tables

B1 Data Collection
B2 Historical AADTs
B3 Growth Rate
B4 Past Forecasts
B5
Design Factors

## APPENDIX C -Traffic Estimates for Shallow Bypass Interchange Options

C1 Scenario S4-ABDE
C2 Scenario S4-ABCE
C3 Scenario S3-ABE
C4 Scenario S2-AE
C5 Scenario P3-ABD
C6 Scenario P2 - AD

## APPENDIX D - Detailed Traffic Counts

D1 Data Collection Summary
D2 Count Location Maps
D3 $\quad 13-\mathrm{Hr}$ and $24-\mathrm{Hr}$ Turn Movement Summary
D4 Tube Counts
D5 Turn Movement Counts

## APPENDIX E - TransCAD Model Files

E1 2015 Model Files
E2 2040 Model Files

### 1.0 INTRODUCTION

The North Carolina Department of Transportation (NCDOT) is currently studying alternatives for a US 70 bypass highway around the City of Kinston, North Carolina as part of the Transportation Improvement Project (TIP) project number R-2553. As part of the alternatives analysis process, traffic forecasts are needed for each alternative to evaluate the feasibility and operations of the proposed bypass alternatives. Parsons Brinckerhoff (PB) was retained to assist NCDOT in developing traffic forecasts for the TIP project R-2553.

This Traffic Forecast Technical Memorandum documents the travel demand modeling and forecasting methodologies and assumptions used in developing future year traffic forecasts for each of the proposed alternatives.

### 1.1 Project Location

The City of Kinston is located in eastern North Carolina in the northwestern section of Lenoir County along US 70 (see Figure 1). It is located approximately 1.5 hours' drive ( 80 miles) east from Raleigh and is situated between two major cities: Goldsboro to the west and New Bern to the east. The City of Kinston is also the home of the Global TransPark, a combined airport (Kinston Regional Jetport at Stallings Field) and industrial complex developed by the State of North Carolina.

### 1.2 Roadway Network

Figure 1 shows that the City of Kinston is currently served by five major US and NC highways which pass through the center of the town: US 70, US 258, NC 11, NC 55, and NC 58. In addition, the C.F. Harvey Parkway, locally signed as NC 148, provides an east-west highway along the northern parts of the City of Kinston and serves as the primary access to the Global TransPark. The City of Kinston and Lenoir County are included in NCDOT Division 2. Specific features of the roads in the study area are described next.

- US 70 is a four-lane major east-west highway providing vital link to eastern North Carolina by providing connections to Raleigh and I-95 to the west, as well as multiple municipalities in eastern North Carolina including Goldsboro, La Grange, Kinston, Dover, and New Bern. The posted speed limit along US 70 ranges from 55 miles per hour ( mph ) on the western and eastern approaches to 45 mph within Kinston. The roadway has multiple traffic signals and provides access to retail and other developments within Kinston. US 70 is classified as 'Rural Principal Arterial' outside of the Kinston city limits and as 'Other Principal Arterial' within the Kinston urbanized area. US 70 is identified on NCDOT's Strategic Highway Corridor Vision Plan as a future freeway from Raleigh to Morehead City. For this entire distance, the US 70 corridor is being studied separately as part of the US 70 Corridor Study (also referred to as Super 70).
- US 258 is primarily a two-lane north-south highway providing regional connections to Jacksonville to the south and Farmville and Tarboro to the north. Through the City of Kinston, US 258 provides
access to US 70 from the south. US 258 has a wider five-lane cross-section north of US 70, and serves as the primary existing access between US 70 and the Global TransPark. It is posted at 45 mph speed limit. US 258 is classified as 'Rural Minor Arterial' outside of the Kinston city limits and 'Other Principal Arterial' within the Kinston urbanized areas. US 258 is identified on NCDOT's Strategic Highway Corridor Vision Plan as a future expressway south of US 70 to NC 24.
- NC 11 is a two-lane rural road providing access to I-40 to the southwest in Duplin County. North of Kinston, NC 11 is a multi-lane highway providing access to Greenville, located 30 miles north-east of Kinston. Within Kinston, NC 11 is a multi-lane roadway that crosses US 70 and crosses through downtown. NC 11 is classified as 'Rural Major Collector' outside of the Kinston city limits and as 'Other Principal Arterial' within the Kinston urbanized areas. NCDOT's Strategic Highway Corridor Vision Plan identifies NC 11 as a future freeway from I-40, continuing north through Kinston (on new alignment in downtown) to US 264 west of Greenville.
- NC 55 is a two-lane rural highway providing east-west access in eastern North Carolina. Within Kinston, NC 55 merges with NC 11 through the downtown area. NC 55 is classified as 'Rural Major Collector' outside of the Kinston city limits. Within the Kinston urbanized areas, NC 55 is classified as 'Minor Arterial' south and west of downtown and as 'Other Principal Arterial' within downtown and to the northeast.
- NC 58 is a two-lane state road, providing north-south connection in Kinston. NC 58 is classified as 'Rural Major Collector' outside of the Kinston city limits. Within the Kinston urbanized areas, NC 58 is classified as 'Other Principal Arterial' in the downtown area and as 'Minor Arterial' north of downtown.
- The C.F. Harvey Parkway (NC 148) provides an east-west link in northern Kinston from US 70 on the west to NC 58. It is comprised of two main sections. Between US 70 and US 258, the recently constructed Parkway extension is a four lane freeway with interchange access at both US 70 and US 258. Between US 258 and NC 58, the Parkway is a four lane divided highway section with traffic signals along major intersections, including the intersections which serve as the primary access points to the Global TransPark. This roadway is classified as 'Other Principal Arterial.'

Figure 1. Study Area Map


### 2.0 PROJ ECT DESCRIPTION

The purpose of the TIP project R-2553 is to evaluate highway bypass alternatives that would provide an alternate option to using the existing US 70 through the City of Kinston. The proposed bypass is intended to serve as a major diversion route for motorists traveling east-west on US 70. In addition to the bypass alternatives, a non-bypass alternative of upgrading US 70 between La Grange and Dover to a freeway is also to be evaluated.

### 2.1 History

The Kinston Bypass has been considered by NCDOT in previous forecasts and studies for projects in and around the City of Kinston. The following forecasting projects were reviewed as part of the current forecasting efforts:

- North Carolina Global TransPark (GTP) Study - May 1996
- R-2719A C.F. Harvey Parkway (formerly Crescent Road) - June 2004
- Kinston Eastern Loop/NC 11 Relocation (FS-0802) - May 2008
- US 70 Strategic Highway Corridor Study (including US 70 at NC 11NC 55 Feasibility Study)
- US 70 Kinston Bypass (R-2553) - July 2009
- R-2554 Goldsboro Bypass (Public Hearing Map) - project completed May 2016
- US 70 Kinston Bypass (R-2553) - July 2012

In addition, the Kinston Comprehensive Transportation Plan (CTP), adopted in February, 2008, provides future regional forecasts assuming multiple transportation projects identified for the area. Many of these projects are not included in the financially feasible network identified for the current Kinston Bypass forecast.

A review of these forecasts was conducted and compared with model runs of the latest Kinston Area regional demand model using TransCAD software. This demand model was initially developed by NCDOT in April 2012 and has been used as part of the latest forecast.

Note that there is general decrease when comparing Kinston forecast before 2012 and after. This reduction in future traffic is primarily a result of reevaluation of expected land use growth in the Kinston area including the Global TransPark that was made as part of the 2012 forecast. A detailed comparison between the older forecasts and the updated analysis is provided in Appendix B page B4 of this report.

### 2.2 Fiscal Constraint

The City of Kinston is not part of any Metropolitan Planning Organization (MPO). Consequently, the current Kinston study area is considered a non-MPO area, and thus not subject to the federal requirement of preparing a fiscally-constrained transportation plan. Consequently, specific projects assumed as part of the current forecasting efforts were determined based on consultation with NCDOT by assuming that only projects identified in the current NCDOT STIP were part of the fiscally constrained network. These future roadway projects and other assumptions are outlined in Section 4.2.

### 2.3 Alternatives

As part of the current traffic forecasting efforts, eleven US 70 Bypass Build alternatives were identified in addition to Upgrade US 70 and No-Build alternatives. For each alternative, traffic forecasts were prepared for two analysis years - 2040 design year conditions, and 2015 existing year conditions.

### 2.3.1 Description of Bypass Alternatives

Figure 2 shows the conceptual location/alignment of the alternatives being studied by NCDOT (received as part of the NCDOT Forecast Request along with a conceptual graphic illustration for each alternative). Table 1 lists the alternatives examined for this forecast. Appendix A also provides the forecast for each alternative which can be reviewed for a more detailed examination of interchange connections and alignments. In all cases, it should be recognized that the specific alignments are preliminary and subject to change as part of the environmental analysis process. Also note that all of the Bypass alternatives examined as part of this forecast represent southern bypasses of Kinston.

Table 1. Bypass Alternatives Evaluated
\(\left.$$
\begin{array}{|c|c|c|}\hline \begin{array}{c}\text { Forecast } \\
\text { Scenario }\end{array} & \begin{array}{c}\text { Alternatives } \\
\text { (See Detailed Study Alternatives Map) } \\
\text { No Build }\end{array} & \begin{array}{c}\text { Alignment } \\
\text { Color }\end{array}
$$ <br>
\hline 1 \& Alternative 1 - Upgrade Existing US 70 to freeway \& Black <br>
\hline 2 \& Alternative 1S (Scenario S5) - Shallow Southern Bypass with 5 <br>

interchanges\end{array}\right]\)| Orange |
| :---: |
| 3 A |

Note: Two variations are examined for Alternatives 1S - Shallow Southern Bypass. The differences between scenarios S5 and P4 include which crossroads have interchanges as well as the manner in which the existing US 70 ties into the Shallow Bypass on the east.

For all alternatives, it is assumed that 70 mph posted speed would be provided. This includes improvements to existing roadway segments including the existing US 70 and C.F. Harvey Parkway. Also note that for all alternatives it is assumed that the 70 mph freeway operations will apply for the entire section from the US 70/NC 903 interchange in LaGrange east through Kinston to the US 70/NC 41 interchange south of Cove City. For alternatives that begin or end within these boundaries, it is assumed that US 70 would be upgraded on the eastern and western approaches to the proposed Bypass alignments.

### 2.3.2 Comparison of Interchange Access Locations

Each of the twelve alternatives (in addition to the No Build) has specific interchange locations identified and assumed for the traffic forecast. As part of the forecast, turn movements are developed for each of these locations. For comparison purposes, the specific interchange locations are identified in Table 2.

Two forecasts have been developed for the Alternative S-1 Shallow Bypass. Both alternatives assume that interchanges are provided at (A) the western tie-in with existing US 70, (B) NC $11 / 55$, (C) US 258, and (D) NC 58. The designation of " P " and " S " refers to the type of interchange connection at ( E ) the eastern tie-in with US 70. The "S" suffix refers to a full access system interchange at the E location resulting in 5 interchanges (hence the S 5 designation).

Compared with S5, the P4 alignment has only 4 interchange with no interchange access at location E. Note that the P originally referred to partial access at this location, but the final alignment layout assumes that all existing US 70 traffic would access downtown Kinston from the NC 58 interchange (D). To facilitate this movement, US 70 would be realigned to overlap with NC 58 between the existing US 70 and the proposed NC 58 interchange.

Recognizing that fewer interchanges (or a prioritized phasing of interchanges) may be considered, a series of AADT traffic estimates are also prepared and documented in Appendix C examining variations in AADT for the Shallow Bypass alternatives with fewer interchanges.

### 2.3.3 Comparison of Alignment, Travel Time, and Speed of Alternatives

As noted in the text, the length of the alternatives vary with each alternative connecting into US 70 at different locations west and east of Kinston. In order to understand the desire of US 70 traffic to utilize specific bypasses, a comparison of the total length and travel times of each alternative was developed. This comparison takes into account required new alignments as well as the takeoff points tofrom US 70.

Table 2 provides a summary of the assumed length for each alternative assuming the same western tiein point (the NC 903 interchange just south of LaGrange) and eastern termini (the NC 41 interchange south of Cove City) on US 70.

In addition to length, the overview includes an estimate of the travel time from the western and eastern limits. It is assumed that for all of the Bypass alternatives (including Upgrade US 70) 70 mph operating
speeds will be provided. This is compared to the existing US 70 which includes 55 mph and 45 mph sections as well as multiple traffic signals.

As an additional source of information, the travel times were adjusted to reflect the potential impact of signal delays, particularly on US 70 in the No Build scenario. Although US 70 is posted at between 45 mph and 55 mph , it also has a heavy concentration of traffic signals which will introduce delays and reduce average travel speeds. As shown in Table 2, signal delay was assumed to increase travel times (and decrease travel speeds) by approximately 40 percent on the existing US 70 sections. Note that this reduction effectively reduced travel speeds to between 35 and 40 mph on US 70 which still maintains LOS C/D operations on the US 70 arterial sections.

A review of Table 2 indicates the following:

- The No-Build alternative is estimated to require 33 minutes to travel east-west through the study area if signals are not considered. Note, however, that with a 40 percent delay related to traffic signals during the peak congested periods, this estimate increases to 46 minutes. The existing US 70 , Alt 1 and Alt 1 S are the shortest alignments through the study area at about 28.7 miles.
- The Upgrade US 70 alternative, Alt 1, reduces east-west travel time ( 25 minutes) compared with the No-Build ( 33 minutes free flow / 46 minutes with assumed signal delays). It is assumed that in Alt 1 US 70 is improved to 70 mph including the construction of interchanges.
- The new alignment Bypass alternatives provide east-west travel time (25-28 minutes) through Kinston. The higher speeds on the Bypass make the Bypass the preferred route for east-west through traffic as compared with existing US 70 (No Build alternative) during both uncongested and congested periods.

Note that the above analysis was not used directly in the forecasting process. Instead these observations were utilized in comparing the reasonableness of the TransCAD model runs for each alternative.

Figure 2. Kinston Bypass Alternatives


Yellow Alignments-Alternatives 31 \& 32



Kinston Bypass (R-2553) Traffic Forecas
Technical Memorandum - November 7, 2016

Table 2．Proposed Interchanges，Length and Travel Time along Bypass Alternatives

| ALT | INTERCHANGES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | $\begin{aligned} & \text { DISTANCE } \\ & (\mathrm{mi}) \end{aligned}$ |  |  | $\begin{aligned} & \text { TRAVEL } \\ & \text { TIME } \\ & \text { (min) } \end{aligned}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \text { ®ু } \\ & \text { ù } \\ & \text { n } \end{aligned}$ |  |  |  |  | $\begin{aligned} & \stackrel{\infty}{\sim} \\ & \stackrel{\sim}{\Omega} \end{aligned}$ | $\begin{aligned} & \text { g } \\ & \stackrel{y}{0} \\ & \stackrel{0}{0} \\ & 0 \end{aligned}$ | $\begin{aligned} & \text { in } \\ & \text { 亿 } \\ & Z \end{aligned}$ | $\underset{Z}{7}$ | $$ | $\begin{aligned} & \infty \\ & \text { ®ٌ } \\ & \text { Z } \end{aligned}$ | $\begin{aligned} & \widetilde{Z} \\ & \stackrel{y}{0} \\ & \stackrel{y}{4} \\ & \stackrel{y}{w} \\ & \vdots \end{aligned}$ | $\begin{aligned} & R \\ & i n \\ & \tilde{0} \\ & 0 \end{aligned}$ |  | $\begin{aligned} & 7 \\ & \overleftarrow{4} \end{aligned}$ | $\begin{aligned} & \text { 픙 } \\ & \end{aligned}$ | $\begin{aligned} & \text { ® } \\ & 0 \end{aligned}$ |  | $\stackrel{\text { II }}{\stackrel{0}{0}}$ | $\begin{aligned} & \text { R } \\ & \text { in } \end{aligned}$ |  | П |
| No Build | x |  |  |  |  |  |  |  |  |  |  |  |  |  | x |  | 28.7 | － | 28.7 | 46.3 | － | 46.3 |
| Alt 1 | X | X |  | X | x | X | x |  | x | X | x | x | X |  | x | 10 | 28.7 | － | 28.7 | 24.6 | － | 24.6 |
| Alt 1S（S5） | x | X |  | x | x |  |  |  | x | X | x | x | X | X | x | 9 | 19.5 | 9.2 | 28.7 | 16.7 | 7.9 | 24.6 |
| Alt 1S（P4） | x | X |  | x | x |  |  |  | x | X | x | x | X |  | x | 8 | 19.5 | 9.2 | 28.7 | 16.7 | 7.9 | 24.6 |
| Alt 11 | x | X |  | X | X |  |  |  | X | X | X | X |  | X | X | 8 | 14.1 | 16.5 | 30.6 | 12.1 | 14.1 | 26.2 |
| Alt 12 | X | X |  | X | X |  |  |  | X | X | X |  |  | X | X | 7 | 18.0 | 13.1 | 31.1 | 15.4 | 11.2 | 26.6 |
| Alt 31 | X | X | X |  | X |  |  |  | X | x | X | x |  | X | X | 8 | 12.2 | 17.3 | 29.5 | 10.5 | 14.8 | 25.3 |
| Alt 32 | x | X | X |  | x |  |  |  | X | x | X |  |  | x | X | 7 | 16.0 | 13.6 | 29.6 | 13.7 | 11.7 | 25.4 |
| Alt 63 | x | X | X |  | X |  |  |  | x | X | X |  |  | X | X | 7 | 16.0 | 13.7 | 29.7 | 13.7 | 11.7 | 25.4 |
| Alt 65 | x | X | X |  | x |  |  |  | x | X | X | x |  | X | x | 8 | 12.5 | 17.1 | 29.6 | 10.7 | 14.7 | 25.4 |
| Alt 51 | x | x | X |  |  |  |  | x | X | X | x | X |  | x | x | 8 | 11.1 | 19.0 | 30.1 | 9.5 | 16.3 | 25.8 |
| Alt 52 | x | X | X |  |  |  |  | X | X | X | X |  |  | X | X | 7 | 14.8 | 15.5 | 30.3 | 12.7 | 13.3 | 26.0 |
| Alt 35 | X | X | X |  |  |  |  | X | X | X | X |  |  | X | X | 7 | 14.8 | 18.1 | 32.9 | 12.7 | 15.5 | 28.2 |
| Alt 36 | x | X | X |  |  |  |  | X | x | X | X | x |  | X | x | 8 | 11.1 | 21.4 | 32.5 | 9.5 | 18.3 | 27.8 |

Notes：
1．Above analysis assumes the same western（NC 903 interchange in LaGrange）and eastern（NC 41 interchange south of Cove City）termini．
2．For No build，travel time includes $40 \%$ delay due to signals on existing US 70 ．

### 3.0 DATA COLLECTION

The first step in traffic forecasting involved collection of existing traffic data in the study area. Traffic data was collected between April 14, 2015 and May 15, 2015 when the schools were in session. Data collection included $48-\mathrm{hr}$ class counts and turn movement counts.

A map showing location of traffic counts is included in Appendix D.

### 3.1 Classification Counts

Daily volume and classification counts were collected at 21 locations in the study area. The data were collected according to NCDOT guidelines and covered at least 48 -hour period during weekdays. Appendix D provides a list of locations where classification counts were collected.

### 3.2 Turn Movement Counts

Turn movement counts (TMC) were collected at 36 locations. TMC data were collected according to NCDOT guidelines and covered a 13-hour period during weekdays from 6 AM to 7 PM . For each of the intersections, truck volumes were counted on one approach. At two locations along US 70, only turn volumes were counted.

The intersections where turning movement counts were collected are shown in Appendix D.

### 3.3 Convert Class Counts to AADT

The 48 hour class counts were converted to AADT according to NCDOT Traffic Survey Group (TSG) guidelines. Computing the AADT involved the following steps:

- The 48 -hour class counts were averaged over the two day period to compute the 24 -hr average daily traffic (ADT) count.
- A seasonal adjustment factor was applied to the above $24-\mathrm{hr}$ ADT counts to convert them to AADT. The seasonal factors were obtained from NCDOT Traffic Survey Group's Seasonal Factor spreadsheet. The count date, ATR group and seasonal adjustment factor used are shown in Appendix B-B1.
- The estimated AADTs were rounded to nearest hundreds.


### 3.4 Convert TMC to Quadrant Volumes

The turn movement counts were the primary source for estimating quadrant volumes at intersections. A two-step process was used to estimate quadrant volumes from turn movement counts. First, 13-hr mainline and quadrant volumes were calculated for intersections where counts were available. Second, a factor was applied to estimate $24-\mathrm{hr}$ AADT equivalent volume. The factor was based on the ratio of estimated AADT from adjacent class counts to 13-hr mainline volumes from turn movement count. Note
that seasonal adjustment is already included in AADT's estimated from class count, and hence reflected in the conversion factor.

### 3.5 Comparison with Historic AADTs

Historic AADTs were obtained from 1994 through 2014 for key locations in the study area. AADTs are compared to counts collected for the projects, and to the base year No Build (BY NB 2015) forecast in Appendix B - Table B2. A historical AADT map for key locations showing NCDOT published AADTs from 2010 through 2014 is shown in Figure 3.

Figure 3. Historical AADT Map


### 4.0 TRAVEL DEMAND MODEL

This Kinston Travel Demand model was a key tool in preparing the forecasts for this project. NCDOT TPB commissioned CDM Smith to develop the Kinston Travel Demand Model based on NCDOT's Small Area Model guidelines. The final model along with model validation and technical memorandum were completed in April 2012. The model utilizes TransCAD platform and has two scenarios:

- BY 2011 - the validated Baseline Year 2011, and
- FY 2040 - Future Year 2040

A model review was conducted early in the process to identify and document key assumptions and model outputs.

### 4.1 Model Review

The initial step in applying the model was to examine and compare the No Build model operations and results, comparing them with previous traffic forecasts and expected land use growth rates to verify the reasonableness of the model. This required refining the No Build network, comparing to previous forecasts, and then assessing reasons for discrepancies.

### 4.1.1 Land Use Assumptions and Population Trend

The development of land use projections was not part of the traffic forecast process. Instead it was conducted separately as part of the planning process during which NCDOT coordinated with local planners in Lenoir County, Kinston, and with the Global TransPark to develop population and employment projections. Table 3 provides a comparison of base year (2011) and future year (2040) data from the model. Data from North Carolina State Data Center (NC SDC) shows that population has essentially been flat in the past 35 years and a similar trend is projected for the next 30 years.

As shown in Table 3, the model anticipates an annual increase in population through 2040 to be approximately 0.57 percent. This is higher than the State Data Center projections of very slow (actually negative growth) as shown in Table 4. This finding was discussed with the NCDOT planners and it was revealed that one of the basic assumptions agreed to with local planners was that the period from 2000 to 2010 has been a very low growth period, but that the 10-year period from 1990 through 2000 was more representative of growth that could occur and be maintained in the area, especially given the presence of the Global TransPark both in terms of infrastructure that is now in place and planned for future. Nevertheless, 0.57 percent increase in population is a relatively modest growth rate.

Table 3. Socio-Economic Data from Model

|  | Population | Employment |
| :---: | :---: | :---: |
| BY 2011 | 43,934 | 29,459 |
| FY 2040 | 51,749 | 38,626 |
| Change | 7,815 | 9,167 |
| Change (\%) | $18 \%$ | $31 \%$ |
| Annual Growth Rate | $\mathbf{0 . 5 7 \%}$ | $\mathbf{0 . 9 4 \%}$ |

Table 4. Population Estimates and Projections for Lenoir County

| Year | Population <br> Estimate | Year | Population <br> Projection |
| :---: | :---: | :---: | :---: |
| 1980 | 59,819 |  | 2015 |
| 1990 | 57,274 |  | 58,780 |
| 2000 | 59,573 | 2020 | 58,533 |
| 2010 | 59,454 | 2025 | 58,282 |
| 2014 | 58,830 | 2035 | 58,034 |

Source: North Carolina State Data Center

### 4.1.2 Global TransPark Employment Assumptions

Within the study area, the Global TransPark is a key driver of traffic demand in the region. Included within the total employment estimate in Table 5 is the observation that 63 percent of the anticipated increase in employment is industrial employment focused at and near the Global TransPark. To examine the GTP growth, six zones were selected from the regional demand model that overlapped the Global TransPark boundaries. Note that in some cases, the employment may not be exclusively related to the GTP, but for estimation purposes the zones are a valid representation.

As shown in Table 5, the estimated employment growth rate in the GTP zones is 4 percent per year. This is a strong rate of growth and would drive increases in traffic for the region. This growth rate was identified by NCDOT in close consultation with GTP staff and local planners and provides a reasonable assumption for local growth.

Note, however, that these 2040 employment projections are substantially lower than estimates used in the previous 2030 Kinston Model as well as previous forecasts. The older version of the model was based on an estimation of 25,000 employees in the GTP. In contrast, the updated 2010 model anticipates 6,718 employees in the Global TransPark zones by 2040. These revised assumptions for future (2040) employment were cooperatively developed by NCDOT with local input from the Global TransPark and Kinston area planners. In the process, it was recognized and acknowledged that these projections were much lower than previous estimates.

Table 5: Total Employment for GTP TAZs in BY 2011 and FY 2040

| Model TAZ <br> (GIP) | BY 2011 | FY2040 | Change | Change <br> (\%) | Annual <br> Growth Rate |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 122 | 11 | 113 | 102 | $927 \%$ | $8.4 \%$ |
| 125 | 1,338 | 1,338 | 0 | $0 \%$ | $0.0 \%$ |
| 127 | 10 | 103 | 93 | $930 \%$ | $8.4 \%$ |
| 131 | 1 | 10 | 9 | $900 \%$ | $8.3 \%$ |
| 132 | 474 | 4,373 | 3,899 | $823 \%$ | $8.0 \%$ |
| 133 | 349 | 781 | 432 | $124 \%$ | $2.8 \%$ |
| Total | $\mathbf{2 , 1 8 3}$ | $\mathbf{6 , 7 1 8}$ | $\mathbf{4 , 5 3 5}$ | $\mathbf{2 0 8 \%}$ | $\mathbf{4 . 0 \%}$ |

### 4.1.3 Review of Total Trip Volumes

In evaluating the new TransCAD regional demand model, an examination of trip volumes and patterns was also reviewed to assess the applicability of the model to the Kinston Bypass forecast. This included an examination of local trips (trips beginning and ending within the model area), internal/external trips (trips having one endpoint within the model area and one external to the area), and through trips.

As shown in Table 6, the expected annual growth rate in trips is approximately 1.1 percent. This includes an annual growth rate of 0.8 percent for local trips which reflects the anticipated 0.57 percent increase in population and 0.94 percent increase in employment shown in Table 3. As per the travel demand model, the majority of traffic growth on US 70 is attributable to "Through trips" (external to external trips) rather than growth inside the model area.

Table 6. Total Trips in Base Year 2011 and No Build 2040 Models

| TripType | $\mathbf{2 0 1 1}$ <br> Model <br> Trips | $\mathbf{2 0 4 0}$ <br> Model <br> Trips | Percent <br> Inarease | Annual <br> Growth <br> Rate |
| :---: | :---: | :---: | :---: | :---: |
| Local Trips | 140,567 | 170,125 | $21.0 \%$ | $0.8 \%$ |
| Internal-External | 52,432 | 68,868 | $31.4 \%$ | $1.2 \%$ |
| Through Trips | 22,569 | 35,533 | $57.4 \%$ | $2.0 \%$ |
| Total Trips | 215,567 | 274,526 | $27.4 \%$ | $1.1 \%$ |

### 4.2 Base Year and No Build Network Modifications

The "No Build" alternative includes roadway improvements/ projects that have been planned or committed to by NCDOT or that can reasonably be expected to be in place by the design year. Since Kinston is a non-MPO area, a formal fiscally constrained plan has not been identified. After a review of NCDOT's transportation plans and consultation with NCDOT staff, it was agreed that the projects identified in Table 7 would be updated for the No Build alternative. Therefore, the following roadway projects were updated in the model to develop a base year and future year No-Build roadway network.

Table 7. Base Year and Future Year Network Modifications

| Project | Desaription | In Base <br> Year <br> (2011) | In Future <br> Year <br> (2040) |
| :---: | :--- | :---: | :---: |
| NC 148 C.F. <br> Harvey <br> Parkway <br> Extension <br> (R-2719A) | This project was completed in 2013. It extended C.F. Harvey Parkway as a <br> four-lane freeway facility between US 70 and US 258. A section of US 70 was <br> also realigned as part of this project | Yes <br> (Added) | Yes <br> (M odified) |
| NC 148 C.F. <br> Harvey <br> Parkway NE <br> Extension <br> (R-5703) | This project was included in the May 2015 STIP as a feasibility study. The <br> proposed project includes construction of a new four-lane divided facility <br> northeast of Kinston extending the existing C.F. Harvey Parkway from NC 58 <br> to NC 11. | No <br> (NA) | Yes <br> (Added) |
| Carey Road <br> Extension <br> (U-3618) | This proposed four-lane local road providing an additional east-west route <br> through western Kinston would extend Carey Road from Rouse Road <br> westward to US 258 Hull Road. It was identified for funding as part of <br> previous STIP (2012) but is not included in latest STIP (M ay 2015). | No <br> (NA) | No <br> (Removed) |

Note: Of the three projects requiring network modifications, only the R-5703 project is being added to the 2040 fiscally constrained network as an anticipated project.

### 4.3 Development of Build Networks

The 2040 Future Year TransCAD model was used as the starting point for the development of twelve future Build alternatives for the Kinston Bypass. Figure 4 illustrates the network coding of these alternatives on a single network. The primary assumptions used in network coding of the alternatives included:

- New alignment sections were coded at 70 mph free-flow speed.
- A bridge penalty of one-half minute was added to the new alignment bridge crossing at the Neuse River. This was done in order to be consistent with base year and future year No Build model where a bridge penalty is being used at all links crossing Neuse River.
- Existing sections of US 70 that were to be utilized as part of a Bypass alternative were assumed to be converted to 70 mph freeway sections. This includes the sections of US 70 between the US 70NC 903 interchange to the western Bypass takeoff point as well as the section of US 70 from the eastern Bypass merge point back with US 70 to the US 70/NC 41 interchange.
- For the sections of US 70 being upgraded to a 70 mph freeway, interchanges were added as specified for each roadway. This included consolidating of other local roads and centroid connectors to remove access directly to US 70 or the C.F. Harvey Parkway.
- The alignments were coded as provided by the NCDOT's Project Development and Environmental Analysis (PDEA) branch in the forecast request as shown in Figure 2.
- Each bypass network was modeled separately for traffic assignments in 2011 and 2040. The 2011 model results were adjusted to prepare the 2015 existing forecasts.

Figure 4. TransCAD Model Network with Bypass Alternatives


### 5.0 DEVELOPMENT OF PROJ ECT LEVEL TRAFFIC FORECASTS

The study required development of project level traffic forecasts with balanced daily traffic turning movements per NCDOT standards. Traffic forecasts were developed for No Build conditions and twelve Build alternatives for Base Year 2015 and Horizon Year 2040.

A key component of developing the forecasts was utilizing the TransCAD model and the model forecasted traffic volumes as described in Section 4.0. In addition to the travel demand model output, other inputs were used including previously developed forecasts, traffic data collected for the project, and engineering judgment.

Beyond the review of the model results for estimating average daily traffic, other critical steps include the development of balanced turn movement and the estimation of traffic factors (including design hour volume percentages, directional splits, and truck percentages). The following sections examine these elements and provide additional insights into the specific forecasts for each alternative.

### 5.1 Development of Mainline Volumes

Mainline volumes were estimated for the alternatives using a combination of methods. A key input into the process, however, was utilizing the travel demand models identified in Section 4.0. The overall 2040 trip table was assigned to the networks devised for each alternative. The model results were compiled and compared to observe both similarities and differences between the alternatives. Variations in the model results were reviewed and analyzed in terms of the expected differences in travel patterns including alternative routes for trips to access various locations in Kinston north of US 70.

An initial review of screenlines was conducted to verify overall trip assignment was reasonable between alternatives. This included detailed review of the diversion from the existing US 70 to the proposed Bypass alternatives and verification that the general trend was that the closer and shorter Bypass alternatives diverted higher volumes from US 70 to the new Bypass. Section 5.5 documents the screen lines from the final adjusted forecasts.

Using the base volumes, turn movement balancing was conducted as outlined in Section 5.2 The balancing was adjusted to provide consistency between alternatives on both the existing US 70 and the proposed Bypass alignments.

### 5.2 Development of Turn Movements

The traffic forecasts developed for this project consisted of daily traffic volumes and turn movements for the roadway network. Specifically, turn movements have been developed at multiple intersections along the existing US 70 corridor and on the proposed bypass alternatives.

The turn movements were developed by first reviewing the daily volume projections from the TransCAD model along US 70, the critical side roads on US 70, and the Bypass alternatives. These volumes were
utilized as a starting point for balancing turn movements. Turn movements were estimated utilizing an iterative process requiring multiple steps.

### 5.3 No Build Alternative

For the No-Build alternative, the existing turn movements served as the primary source of data. Using the growth rates predicted for each roadway link from the travel demand model between 2012 and 2040, future turn movements were estimated at each intersection. The next step required balancing of the turn movements to verify that the traffic entering and exiting the intersection matched the turn movements. In some locations, the daily traffic on roadway sections between two intersections was adjusted so that traffic volumes would be balanced between intersections. This typically occurred when a driveway, roadway, or intersection caused a change in traffic volumes between two turn movement locations.

### 5.4 Build Alternatives

For the 2040 Build alternatives, additional steps were used in estimating turn movements. Instead of using the existing counts as a starting point, the 2040 No-Build turn movements were used as a base. A spreadsheet was utilized to iteratively adjust the volumes of turn movements as well as intersection approaches. In general, the provision of Bypass alternatives, reduced the traffic volumes on US 70 through the study area.

For each of the Build alternatives, a freeway type facility is planned along US 70 between NC 903 on the west and NC 41 on the east. Alternative 1 (Widen Existing) assumes that existing at-grade intersections will be modified and/or closed from the existing configuration on US 70. The revised access patterns were examined to estimate future turn movement volumes using existing data as a base, but taking into account the rerouting of trips based on the proposed interchange access points.

For the southern bypass alternatives on new alignment, the portion of the route along existing US 70 (i.e. the eastern and western approaches to the new alignment sections) also is planned to have freeway type access only at interchanges. Therefore, similar to Alternative 1, future turn movements were estimated recognizing that the existing access pattern would be changed resulting in diversions and rerouting of trips from the existing travel patterns.

In addition to the existing roadway sections, traffic volumes and turn movements were assigned to each Bypass alternative. In general, the TransCAD assignment was used to assign daily traffic volumes to the roadway links. More detailed refinements were applied utilizing data from existing turn movement counts as appropriate. Turn movements were estimated and balanced at the tie-in points of the proposed Bypass alignments with US 70.

Once intersections were balanced between themselves for each alternative, an additional step required comparing turn movements for each alternative to each other. In several cases, the bypass alternatives tested were similar and would be expected to have similar traffic patterns in some areas of the network.

As a final step, a quality check was performed to verify that the intersections remained balanced between alternatives.

### 5.5 Screenline Comparison of Alternative Forecasts

A check for each of the alternatives was performed to verify whether total trips across screenlines were consistent between alternatives and that no Alternative forecast had either reduced or increased the total traffic volumes inadvertently. Note that due to intricacies of the roadway network it is anticipated that some volumes differences (less than 10 percent) could occur between alternatives. In performing the screenline analysis, the forecast volumes were identified for three separate screenlines as shown in Figure 5. Table 8 summarizes the screenline analysis of the 2040 alternative forecasts.

- Screenline A - Measured east-west flow generally following the Neuse river boundary on western side.
- Screenline B - Measured east-west flow on the eastern side of the model area.
- Screenline C - Measured north-south flow between C.F. Harvey Parkway and NC 58 north of existing US 70.

A review of the screenlines indicates the following:

- At all three screenlines, volumes are generally consistent across all alternatives and within + $10 \%$ of No Build.
- At screenlines A and B, total volume increases compared to No Build with the bypass extending outward as relatively higher local traffic utilize the bypass.
- Build Alternatives with direct connection to NC 148 (CF Harvey Parkway) have higher volumes than No Build for Screenline C.

Overall, the screenline analysis provides a solid check that the 2040 traffic forecast is applicable for a comparison of the alternatives.

Figure 5. Screenlines for Comparing Forecasts


Table 8. Screenline Analysis of 2040 Forecasts

| ALTERNATIVE | SCREENLINE A |  |  |  | SCREENLINE B |  |  |  | SCREENLINE C |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | US 70 | Bypass | Total | Increase from No Build | US 70 | Bypass | Total | Incre ase from No Build | $\begin{aligned} & \text { NC } \\ & 148 \end{aligned}$ | $\begin{aligned} & \text { US } \\ & 258 \end{aligned}$ | US 70 Bus | $\begin{gathered} \text { NC 11- } \\ 55 \end{gathered}$ | NC 58 | Total | Change <br> from <br> No <br> Build |
| No-Build | 49,400 | - | 49,400 | - | 29,200 | - | 29,200 | - | 9,400 | 13,200 | 22,400 | 14,200 | 18,800 | 78,000 | - |
| Alt 1 | 47,000 | - | 50,000 ${ }^{1}$ | 1\% | 29,000 | - | 31,000 ${ }^{1}$ | 6\% | 8,600 | 14,400 | 23,800 | 15,000 | 19,200 | 81,000 | 4\% |
| Alt S1 (Var S5) | 22,800 | 29,000 | 51,800 | 5\% | 13,000 | 19,000 | 32,000 | 10\% | 14,400 | 8,200 | 18,000 | 17,200 | 20,000 | 77,800 | 0\% |
| Alt S1 (Var P4) | 21,800 | 30,600 | 52,400 | 6\% | 2,500 | 27,200 | 29,700 | 2\% | 14,400 | 8,400 | 18,000 | 17,200 | 20,000 | 78,000 | 0\% |
| Alt 12 | 28,200 | 24,800 | 53,000 | 7\% | 14,800 | 16,000 | 30,800 | 5\% | 13,800 | 8,400 | 22,000 | 14,000 | 18,800 | 77,000 | -1\% |
| Alt 11 | 28,200 | 24,800 | 53,000 | 7\% | 14,800 | 16,000 | 30,800 | 5\% | 13,800 | 8,400 | 22,000 | 14,000 | 18,800 | 77,000 | -1\% |
| Alt 32 | 28,600 | 24,200 | 52,800 | 7\% | 14,800 | 16,200 | 31,000 | 6\% | 13,600 | 8,800 | 22,000 | 14,000 | 18,800 | 77,200 | -1\% |
| Alt 31 | 28,600 | 24,200 | 52,800 | 7\% | 14,800 | 16,200 | 31,000 | 6\% | 13,600 | 8,800 | 22,000 | 14,000 | 18,800 | 77,200 | -1\% |
| Alt 63 | 27,800 | 25,600 | 53,400 | 8\% | 14,800 | 16,200 | 31,000 | 6\% | 13,600 | 8,600 | 22,000 | 14,000 | 18,800 | 77,000 | -1\% |
| Alt 65 | 27,800 | 25,600 | 53,400 | 8\% | 14,800 | 16,200 | 31,000 | 6\% | 13,600 | 8,600 | 22,000 | 14,000 | 18,800 | 77,000 | -1\% |
| Alt 52 | 32,400 | 18,800 | 51,200 | 4\% | 14,800 | 16,200 | 31,000 | 6\% | 10,000 | 10,600 | 22,000 | 14,000 | 19,000 | 75,600 | -3\% |
| Alt 51 | 32,400 | 18,800 | 51,200 | 4\% | 14,800 | 16,200 | 31,000 | 6\% | 10,000 | 10,600 | 22,000 | 14,000 | 19,000 | 75,600 | -3\% |
| Alt 35 | 37,400 | 13,400 | 50,800 | 3\% | 19,800 | 10,400 | 30,200 | 3\% | 10,000 | 11,000 | 22,000 | 14,000 | 19,000 | 76,000 | -3\% |
| Alt 36 | 37,400 | 13,600 | 51,000 | 3\% | 19,800 | 11,000 | 30,800 | 5\% | 10,000 | 11,000 | 22,000 | 14,000 | 19,000 | 76,000 | -3\% |

Notes:
1: Screenline includes traffic on service road parallel to US 70.

### 6.0 TRAFFIC FACTORS

As part of the evaluation of future traffic projections, analysis of existing traffic factors and characteristics were determined and changes in the future were estimated. Specific data which were estimated for this analysis include design hour percentage (K), directional splits (D), and truck percentages (T). These factors were not derived from the TransCAD demand model.

Utilizing the existing traffic counts described in Section 3.1 and 3.2, the K, D, and T factors for each intersection were evaluated. In reporting the data for each alternative, the factors were mapped on the same sheet as the projected turn movements for each alternative. The factors are summarized using the standard NCDOT forecasting tables in Appendix B.

As part of the design year traffic forecasts, traffic factors were developed for directional splits (D), design hour volume percentages (DHV), and truck percentages. These factors are identified in the traffic forecasts shown in Appendix A. Traffic factors were separately estimated for specific roadways including US 70, the C.F. Harvey Parkway, the proposed Bypass alignments, and cross roads.

The primary sources of data for determining the traffic factors was the existing 2008 traffic counts, additional NCDOT counts, and recently completed NCDOT forecasts in the region (FS-0802, U-3618, and R-2719A). In addition, NCDOT's Traffic Forecast Utility spreadsheet was used to verify that the directional splits and DHV were valid for the peak hour intersection flows.

### 6.1 Directional Splits (D)

The directional splits were estimated based upon existing count data, an evaluation of the impact that bypass alignments may have on travel patterns, recently completed NCDOT forecasts, and engineering judgment. The observed and predicted direction splits were compared for specific roadway links as presented in Table 9.

In general, it was determined that the peak hour directional split was $55-45$ on US 70 west and east of Kinston with a 60-40 split applicable on the section between US 70 Business and NC 58. Other local roadways had varying splits with a peak directional percentage between 60 and 65 percent on most roadways. In general, the existing peak hour data was used to identify existing directional distributions and then adjusted based on the specific Bypass alternative. In general, the Bypass alternatives were estimated to have a 55-45 directional split.

### 6.2 Design Hour Volume Percentage (K)

Traffic data was analyzed to identify the percentage of daily traffic occurring during the peak hour (K). These values were compared with the estimate of design hour volume percentage in other NCDOT forecasts. The comparison of the Design Hour Volume (DHV) percentage is illustrated in Table 9. The forecast percentage was adjusted slightly using engineering judgment and rounding.

In general, it was estimated that the future DHV would be approximately 9 percent on US 70 and either 9 or 10 percent on the bypass alignments. In general, the crossroads with US 70 were estimated to have a 9 percent DHV. Also note that for other local and rural roadways in the study area, the DHV percentage varied between 8 and 10 percent based primarily on existing data.

### 6.3 Truck Percentages

Truck percentages were estimated based upon existing turning movements, classification counts, additional NCDOT classification data collected near the project study area, and past NCDOT forecasts. The truck percentages were estimated separately for dual trucks (buses and single unit trucks) and the larger TTST (tractor-trailer and semi-trailer) trucks. The truck percentages are summarized in Table 9.

In addition, truck percentages were tested to verify similar assumptions for total through trucks and GTP related truck trips between alternatives. In some cases, this resulted in fluctuating truck percentages along the bypass alignments with high truck percentages on low volume links and lower percentages on high volume links. This was estimated to maintain a similar volume of through truck volumes along the bypass despite increases in local traffic on specific segments.

Overall, the base peak hour truck percentages on US 70 were estimated as utilizing the existing classification counts collected along US 70. As shown in Appendix B - B5, the data has a wide range for the US 70 truck counts, but in general the truck percentages are higher on the west and east and lower within Kinston where there is a higher volume of local trips. In general, two primary sources of heavy truck volume were assumed: trucks accessing the GTP along C.F. Harvey Parkway to the north as well as through trucks on US 70 which were assumed to divert to the bypass alignment. On the bypass alignments, the truck percentage varied, often with a higher truck percentage on the lowest volume links reflecting through trucks on the bypass alignment.

Truck percentage on side streets typically reflected existing counts, particularly for the roadways serving the downtown area. Note that the truck percentage for the C.F. Harvey Parkway was increased from existing data to reflect the anticipated buildout scenario with extensive truck traffic activity (in addition to rail and air traffic) serving the GTP.

Note that assumed definitions for dual and TTST trucks followed the current NCDOT truck classification scheme. Specifically, duals were assumed to match Classes 4 through 7 and TTST were assumed to match Classes 8 through 13 of the Federal vehicle classification scheme.

Table 9. Traffic Design Factors - D, K and HV at Key Locations

| Location | D - Directional Distribution |  | K - Peak Hour Factor |  | Heavy Vehicles (Dual / TTST) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Class <br> Count | Selected <br> BY NB <br> Value | Class <br> Count | Selected <br> BY NB <br> Value | Class <br> Count | Selected BY NB <br> Value |
| US 70-West of SR 1334 (Barwick Station Rd) | 55\% | 55\% | 8.0\% | 9\% | 4.3 /8.0 | $4 / 8$ |
| US 70 - West of Pinelawn Cemetery Dr | 54\% | 50\% | 8.1\% | 9\% | $3.8 / 6.5$ | $4 / 6$ |
| US 70 - West of US 70 Bus | 51\% | 55\% | 7.6\% | 9\% | 3.2 /4.9 | $4 / 5$ |
| US 70 - East of US 70 Bus | 53\% | 55\% | 8.0\% | 9\% | $3.6 / 5.2$ | $5 / 6$ |
| US 70 - East of Lenoir CC | 53\% | 55\% | 7.6\% | 8\% | $9.1 / 9.0$ | $6 / 9$ |
| US 70 - East of SR 1313 (Burkett Road) | 50\% | 55\% | 8.0\% | 8\% | $7.0 / 12.0$ | 7/12 |
| C.F. Harvey Parkway - North of US 70 | 61\% | 60\% | 9.7\% | 9\% | 4.8 /8.4 | 5/9 |
| NC 11/55- North of NC 11 and 55 Merge | 61\% | 65\% | 8.0\% | 10\% | $5.6 / 8.1$ | $6 / 7$ |
| US 258 - South of SR 1149 (Joe Nunn Road) | 64\% | 60\% | 8.8\% | 9\% | $7.4 / 5.3$ | $8 / 5$ |
| NC 58 - South of SR 1900 (Collier Loftin Road) | 73\% | 65\% | 8.9\% | 9\% | 3.2 /4.2 | 4/4 |

### 7.0 TRAFFIC FORECASTS

The results of the traffic forecasts are shown in Appendix A - Traffic Forecasts. The traffic forecast sheets include daily traffic volumes on roadway links, turn movement volumes, and other traffic factors. The trip assignment from the TransCAD travel demand model was a critical first step in estimating future volumes on roadways, but additional steps were required to verify and balance the counts.

### 7.1 No Build Alternatives

Results of No Build Alternatives are shown in Appendix A. A review and comparison of the existing and future No-Build projections is consistent with anticipated patterns. A steady increase in traffic volumes is anticipated throughout the area and the major through movements increase at a higher percentage than turn movements along the major corridors.

### 7.2 Build Alternatives

Year 2015 and year 2040 traffic forecast sheets are also included in Appendix A for each of the Build alternatives.

A comparison of the alternatives indicates multiple trends including:

- Each of the bypass alternatives divert traffic from US 70. By 2040, the bypass alternatives result in diversions between $12,000 \mathrm{vpd}$ and $30,000 \mathrm{vpd}$.
- The Improve Existing US 70 alternative results in higher volumes of traffic on US 70 due to an increase in speed on some sections to 70 mph and the elimination of traffic signals. In isolated sections, however, lower volumes are noted due to the closure of specific access points and the inclusion of frontage roads for local access. However, since there is no major alternative to US 70, the model results suggest that trips from other facilities would not substantially divert to use the improved US 70.
- The bypass alternatives are expected to carry between 20,000 and 30,000 vehicles per day (vpd) on the western section (west of US 58) and between 10,000 and 30,000 vpd on the eastern section by the year 2040. A significant portion of this traffic is being diverted from US 70. In these scenarios, the C.F. Harvey Parkway will serve as the primary access to the GTP from the existing US 70.

Note that volumes noted above are approximations. For specifics related to each alternative, refer to the traffic forecasts shown in Appendix A.

### 8.0 TRAFFIC ESTIMATES FOR SHALLOW BYPASS

As noted in Section 2.3.1, two forecasts were developed for the Alternative S-1 Shallow Bypass. Both alternatives assumed that interchanges are located at all practical locations on the Shallow Bypass alignment. Recognizing that fewer interchanges (or a prioritized phasing of interchanges) may be considered, a series of AADT traffic estimates are also prepared and documented in Appendix C.

Six traffic estimates were developed as shown in Table 10. These estimates examine variations in AADT for the Shallow Bypass alternatives with fewer interchanges. It is intended that these estimates can provide additional insights into identifying preferred locations for interchanges on the Shallow Bypass alignment. It is recognized that due to the shorter length of the Shallow Bypass, interchange access may be restricted by either traffic operations related to ramp spacing between interchanges as well as overlap between potential access routes for traffic to from downtown Kinston.

Table 10. Scenarios for AADT Estimates with Shallow Bypass

| Alternative S1 Shallow Bypass |  | Interchange Locations |  |  |  |  | Type of Forecast | $\begin{gathered} 2040 \\ \text { AADT on } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Interchange Variation | \# of <br> Interchanges | $\begin{gathered} \text { A } \\ \text { US } 70 \\ \text { West } \end{gathered}$ | $\begin{gathered} \text { B } \\ \text { NC 11/55 } \end{gathered}$ | $\begin{gathered} \text { C } \\ \text { US } 258 \end{gathered}$ | $\begin{gathered} \text { D } \\ \text { NC } 58 \end{gathered}$ | $\begin{gathered} \text { E } \\ \text { US } 70 \\ \text { East } \end{gathered}$ |  | Shallow <br> Bypass <br> (vpd) |
| S Scenarios - Shallow Bypass with Full Access Interchange at US 70 East |  |  |  |  |  |  |  |  |
| S5-ABCDE | 5 | X | X | X | X | X | Formal Forecast in Appendix A | 29,000 |
| S4-ABDE | 4 | X | X |  | X | X | Estimate in Appendix C | 28,500 |
| S4- ABCE | 4 | X | X | X |  | X | Estimate in Appendix C | 28,800 |
| S3- ABE | 3 | X | X |  |  | X | Estimate in Appendix C | 27,600 |
| S2- AB | 2 | X |  |  |  | X | Estimate in Appendix C | 17,000 |
| P Scenarios - Shallow Bypass with No Access at US 70 East \& Realigned US 70 into NC 58 |  |  |  |  |  |  |  |  |
| P4-ABCD | 4 | X | X | X | X |  | Formal Forecast in Appendix A | 30,600 |
| P3-ABD | 3 | X | X |  | X |  | Estimate in Appendix C | 29,000 |
| P2-AD | 2 | X |  |  | X |  | Estimate in Appendix C | 19,400 |

As part of the estimation process, a screenline analysis was conducted for each scenario. Multiple screenlines were examined, with the two primary screenlines summarized in Table 11. As shown, the screenlines for the Shallow Bypass alternatives with fewer interchanges are within 5 percent of the formal forecasts for the S5-ABCDE and P4-ABCD alternatives.

Table 11. Screenline Summary for Shallow Bypass Estimates

| Roadway | No <br> Build | S5ABCDE | $\begin{gathered} \text { S4- } \\ \text { ABDE } \end{gathered}$ | $\begin{gathered} \mathrm{S} 4- \\ \mathrm{ABCE} \end{gathered}$ | $\begin{gathered} \text { S3- } \\ \text { ABE } \end{gathered}$ | $\begin{aligned} & \text { S2- } \\ & \text { AE } \end{aligned}$ | $\begin{gathered} \mathrm{P4}- \\ \mathrm{ABCD} \end{gathered}$ | $\begin{gathered} \text { P3- } \\ \text { ABD } \end{gathered}$ | $\begin{aligned} & \text { P2- } \\ & \text { AD } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Western Sareenline - Near Mit. Vemon Park Drive |  |  |  |  |  |  |  |  |  |
| US70 | 490 | 222 | 224 | 222 | 234 | 328 | 212 | 220 | 336 |
| Bypass | 0 | 290 | 286 | 288 | 276 | 170 | 306 | 290 | 194 |
| Total | 490 | 512 | 510 | 510 | 510 | 498 | 518 | 510 | 530 |
| Percent of Forecast |  |  | 100\% | 100\% | 100\% | 97\% |  | 98\% | 102\% |
| Eastern Sareenline - Near Meadowbrook Drive |  |  |  |  |  |  |  |  |  |
| US 70 | 372 | 168 | 170 | 190 | 192 | 202 | 148 | 164 | 182 |
| Bypass | 0 | 200 | 198 | 178 | 176 | 170 | 216 | 202 | 194 |
| Total | 372 | 368 | 368 | 368 | 368 | 372 | 364 | 366 | 376 |
| Percent of Forecast |  |  | 100\% | 100\% | 100\% | 101\% |  | 101\% | 103\% |

Notes:

1. AADT volume shown in 100 s to match the estimate format. (eg., 494 refers to $49,400 \mathrm{vpd}$ )
2. Full forecasts for S 5 and P4 Shallow Bypass alternatives are shown in red text.

Review of the Appendix E traffic estimates yielded the following observations:

- In general, the greater the number of interchanges, the higher the traffic volumes on the Shallow Bypass alignment. For the $S$ scenarios, the AADT volume on the western segment range from $17,000 \mathrm{vpd}$ with 2 interchanges (i.e. the eastern and western tie-ins) to 29,000 vpd with 5 interchanges. For the P scenarios, the AADT volume on the western segment range from 19,400 vpd with 2 interchanges (i.e. the eastern and western tie-ins) to $30,600 \mathrm{vpd}$ with 4 interchanges.
- The P scenarios are anticipated to carry slightly higher volumes than the $S$ scenarios. The primary reason is that trips between the western termini and the commercial area just west of US 258 tend to stay on the existing US 70 in the $S$ scenarios (i.e. with US 70 existing remaining in place), but use the Shallow Bypass in the P scenarios (i.e. with the existing US 70 diverted onto NC 58).
- Of the interior interchange locations (i.e. B, C, and D), an interchange at the NC $11 / 55$ connection (i.e. B) results in the highest volume of vehicles on the proposed bypass. An interchange at this
location serves the highest volume of vehicles. The primary movement for vehicles using this interchange are to from the west.
- For the S scenarios (i.e. with an eastern tie-in at US 70), both the US 258 (C) and NC 58 (D) interchange locations have a minimal effect with the overall AADT only increasing by approximately $1,200 \mathrm{vpd}$.
- For the P scenarios, the (D) NC 58 interchange location is required for connectivity. It also serves a higher volume of vehicles since traffic between downtown Kinston and US 70 to the east must use the NC 58 interchange. As with the S scenarios, the US 258 (C) interchange location has a minimal effect with the overall AADT only increasing by approximately $1,200 \mathrm{vpd}$.
- On the estimate for each scenario, the anticipated AADT volumes are illustrated for the existing US 70 section. In general, the higher the volumes on the Shallow Bypass, the lower the volumes are on the existing US 70. A review of the estimates indicates that the lowest future AADT volumes on the existing US 70 will occur on the section between NC 11 and the US 258 intersections. At this location on the existing US 70, the AADT volumes vary from 7,800 vpd to 11,200 vpd compared with 29,600 vpd in the 2040 No Build.

Note that these findings only present traffic forecasting results. Differences in congestion relief, land use impacts, natural resource impacts, and construction costs all must be examined in determining the preferred locations, number, and type of interchanges.

## APPENDIX A

## TRAFFIC FORECASTS

- A1 - Scenario 1-2015 Base Year Existing Conditions
- A2 - Scenario 2-2015 Base Year Build Alternative 1 (Upgrade US 70)
- A3 - Scenario 3A - 2015 Base Year Build Alternative 1S-S5 (Shallow Bypass with 5 interchanges)
- A4 - Scenario 3B-2015 Base Year Build Alternative 1S-P4 (Shallow Bypass with 4 interchanges \& rerouting of existing US 17 to NC58)
- A5 - Scenario 4-2015 Base Year Build Alternative 11
- A6 - Scenario 5-2015 Base Year Build Alternative 12
- A7 - Scenario 6-2015 Base Year Build Alternative 31
- A8 - Scenario 7-2015 Base Year Build Alternative 32
- A9 - Scenario 8-2015 Base Year Build Alternative 35
- A10- Scenario 9-2015 Base Year Build Alternative 36
- A11 - Scenario 10-2015 Base Year Build Alternative 51
- A12 - Scenario 11-2015 Base Year Build Alternative 52
- A13-Scenario 12-2015 Base Year Build Alternative 63
- A14 - Scenario 13-2015 Base Year Build Alternative 65
- A15-Scenario 14-2040 Horizon Year No Build
- A16 - Scenario 15-2040 Horizon Year Build Alternative 1 (Upgrade US 70)
- A17 - Scenario 16A - 2040 Horizon Year Build Alternative 1S-S5 (Shallow Bypass with 5 interchanges)
- A18 - Scenario 16B - 2040 Horizon Year Build Alternative 1S-P4 (Shallow Bypass with 4 interchanges \& rerouting of existing US 17 to NC 58)
- A19-Scenario 17-2040 Horizon Year Build Alternative 11
- A20-Scenario 18-2040 Horizon Year Build Alternative 12
- A21 - Scenario 19-2040 Horizon Year Build Alternative 31
- A22 - Scenario 20-2040 Horizon Year Build Alternative 32
- A23 - Scenario 21-2040 Horizon Year Build Alternative 35
- A24-Scenario 22-2040 Horizon Year Build Alternative 36
- A25-Scenario 23-2040 Horizon Year Build Alternative 51
- A26 - Scenario 24-2040 Horizon Year Build Alternative 52
- A27 - Scenario 25-2040 Horizon Year Build Alternative 63
- A28-Scenario 26-2040 Horizon Year Build Alternative 65


## APPENDIX A 2015 Scenarios







average annual Scenario 3A DAlly traffic

## LEGEND

\#\#\# Vehicles Per Day (VPD) in 100s
Less than 50 VPD
X Movement Prohibited Roadway
Future Interchange Existing Interchange
$\xrightarrow[(\mathrm{d}, \mathrm{t})]{\mathrm{PM}}$
K Design Hour Volume Percentage
PM PM Peak Period
D Peak Hour Directional Split
$\rightarrow \quad$ Indicates Direction of $D$
(d,t) Duals, TT-STs (\%)

BUILD ALTERNATIVE 1S-S5

| TIP: R-2553 | WBS: 34460 |
| :--- | :--- |
| COUNTY: Lenoir | DIVISION: $\mathbf{2}$ |
| DATE: November 7, 2016 |  |
|  | PREPARED BY: Parsons Brinckerhoff |
| LOCATION: US 70 <br> from west of NC 903 in La Grange <br> to east of Old US 70 in Dover |  |
|  | PROJECT: US 70 Kinston Bypass |





## 

Scenario 3B Base Year






















## APPENDIX A 2040 Scenarios








AVERAGE ANNUAL Scenario 16A DAILY TRAFFIC Horizon Year

## LEGEND

\#\#\# Vehicles Per Day (VPD) in 100s

1. Less than 50 VPD

X Movement Prohibited
Roadway
Future Interchange
Existing Interchange

K Design Hour Volume Percentage
$\begin{array}{ll}\text { K } & \text { Design Hour Volu }\end{array}$
D Peak Hour Directional Split
$\rightarrow \quad$ Indicates Direction of $D$
(d,t) Duals, TT-STs (\%)

BUILD ALTERNATIVE 1S-S5
(Shallow Bypass) SHEET 1 OF 2

| TIP: R-2553 | WBS: 34460 |
| :--- | :--- |
| COUNTY: Lenoir | DIVISION: 2 |
|  | DATE: November 7, 2016 |
|  | PREPARED BY: Parsons Brinckerhoff |

PREPARED BY: Parsons Brinckerhoff LOCATION: US 70 from west of NC 903 in La Grange to east of Old US 70 in Dover PROJECT: US 70 Kinston Bypass
























## APPENDIX B

## FORECAST DOCUMENTATION TABLES

- B1 - Data Collection
- B2 - Historical AADTs
- B3-Growth Rate
- B4 - Past Forecasts
- B5 - Design Factors


## B1. DATA COLLECTION TABLE - Tube Count

| Location | Key Map \# | Count Type | Date(s) | County | ATR Group | Seasonal Adjustment Factor | $\begin{aligned} & 2015 \\ & \text { AADT } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| US 70 West of Barwick Station Rd | 101 | 48 hr Count | April 28,29,30, 2015 | Lenoir | 2 | 1.09,1.08,1.02 | 20,600 |
| US 70 West of Pinelawn Cemetery Dr | 102 | 48 hr Count | May 12,13,14, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 22,400 |
| US 70 West of US 70 Business at Hillcrest Road | 103 | 48 hr Count | May 12,13,14, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 36,700 |
| US 70 West of NC 11 | 104 | 48 hr Count | May 5,6,7, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 26,800 |
| US 70 East of Lenoir CC | 105 | 48 hr Count | May 12,13,14, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 14,800 |
| US 70 East of Burkett Road | 106 | 48 hr Count | May 5,6,7, 2015 | Jones | 2 | 1.02,1.03,0.97 | 11,000 |
| C.F. Harvey Parkway North of US 70 | 100 | 48 hr Count | May 13,14,15, 2015 | Lenoir | 2 | 1.03,0.97,0.83 | 2,800 |
| NC 11/55 North of NC 11 and NC 55 Merge | 107 | 48 hr Count | May 5,6,7, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 12,500 |
| US 258 South of Joe Nunn Road | 108 | 48 hr Count | May 5,6,7, 2015 | Lenoir | 2 | 1.02,1.03,0.97 | 7,100 |
| NC 58 South of Collier Loftin Road | 109 | 48 hr Count | April 28,29,30, 2015 | Lenoir | 2 | 1.09,1.08,1.02 | 7,000 |
| Kennedy Home Road South of US 70 | 110 | 48 hr Count | April 15,16,17, 2015 | Lenoir | 1 | 0.98,0.95,0.87 | 1,400 |
| Kennedy Home Road South of Barrus Rd | 111 | 48 hr Count | April 20,21,22, 2015 | Lenoir | 1 | 1.01,0.99,0.98 | 1,500 |
| Innovation Way North of US 70 | 112 | 48 hr Count | April 15,16,17, 2015 | Lenoir | 1 | 0.98,0.95,0.87 | 2,400 |
| Elijah Loftin Road East of Whaley Road | 113 | 48 hr Count | April 28,29,30, 2015 | Lenoir | 1 | 1.09,1.08,1.02 | 600 |
| Cobb Road North of Elijah Loftin Road | 114 | 48 hr Count | April 28,29,30, 2015 | Lenoir | 1 | 1.09,1.08,1.02 | 600 |
| Alexander Rouse Road West of Parker Fork Road | 115 | 48 hr Count | April 28,29,30, 2015 | Lenoir | 1 | 1.09,1.08,1.02 | 200 |
| Clarence Potter Road West of US 258 | 147 | 48 hr Count | April 20,21,22, 2015 | Lenoir | 1 | 1.01,0.99,0.98 | 100 |
| Lenoir Community College Exit, East of NC 58 | 148 | 48 hr Count | April 20,21,22, 2015 | Lenoir | 1 | 1.01,0.99,0.98 | 1,900 |
| Lenoir Community College Entrance, East of NC 58 | 149 | 48 hr Count | April 20,21,22, 2015 | Lenoir | 1 | 1.01,0.99,0.98 | 1,300 |
| Sanderson Way East of Industrial Way | 166 | 48 hr Count | April 16,17,18, 2015 | Lenoir | 1 | 1.02,0.88 | 2,300 |
| Leslie Stroud Road West of NC 11 | 176 | 48 hr Count | April 21,22,23, 2015 | Lenoir | 1 | 0.99,0.98,0.95 | 300 |

Total 21

B1. DATA COLLECTION TABLE - Turn Movement Count

| Location | Key Map \# | Count Type | Date | County |
| :---: | :---: | :---: | :---: | :---: |
| US 70 WB Ramps at NC 903 | 1A | 13 hour TMC | April 14, 2015 | Lenoir |
| US 70 EB Ramps at NC 903 | 1B | 13 hour TMC | April 14, 2015 | Lenoir |
| US 70 at SR 1603 Washington Street | 2 | 13 hour TMC | April 15, 2015 | Lenoir |
| US 70 at Jim Sutton Rd/Willie Measley Rd | 3 | 13 hour TMC | April 15, 2015 | Lenoir |
| US 70 at SR 1334 Barwick Station Road | 5 | 13 hour TMC | April 15, 2015 | Lenoir |
| US 70 at SR 1519 Eason Rd/SR 1324 Kennedy Home Rd | 8 | 13 hour TMC | April 17, 2015 | Lenoir |
| US 70 at SR 1546 Bank School Rd | 9 | 13 hour TMC | April 17, 2015 | Lenoir |
| Sanderson Way at SR 2003 Industrial Dr | 11 | 13 hour TMC | April 16, 2015 | Lenoir |
| US 70 at Pinelawn Cementary Dr/Shopping Center Dr | 13 | 13 hour TMC | April 20, 2015 | Lenoir |
| US 70 at SR 1548 Hill Fram Rd/Sussex St | 14 | 13 hour TMC | April 21, 2015 | Lenoir |
| US 70 at Walmart Dr/Sheffield Dr | 15 | 13 hour TMC | April 22, 2015 | Lenoir |
| US 70 at US 258 North | 16 | 13 hour TMC | April 22, 2015 | Lenoir |
| US 70 at Mt Vernon | 17 | 13 hour TMC | April 23, 2015 | Lenoir |
| US 70 - US 258 BUS at Hillcrest Rd | 20 | 13 hour TMC | April 17, 2015 | Lenoir |
| US 70 at NC 55/11 | 21 | 13 hour TMC | April 24, 2015 | Lenoir |
| US 70 at US 70 BUS / US 258 | 22 | 13 hour TMC | May 6, 2015 | Lenoir |
| US 70 at NC 58/Trenton Rd | 24 | 13 hour TMC | May 5, 2015 | Lenoir |
| US 70 at Lenoir CC | 25 | 13 hour TMC* | May 6, 2015 | Lenoir |
| US 70 at SR 1804 Neuse Rd | 26 | 13 hour TMC | May 7, 2015 | Lenoir |
| US 70 at SR 1903 Whaley Rd | 27 | 13 hour TMC | April 28, 2015 | Lenoir |
| US 70 at SR 1309 Caswell Rd/SR 1002 Wyse Fork Rd | 29 | 13 hour TMC | May 8, 2015 | Jones |
| US 70 at Old Dover Rd | 32 | 13 hour TMC | May 8, 2015 | Jones |
| NC 903 at SR 1324 Jenny Lind Rd | 33 | 13 hour TMC | April 14, 2015 | Lenoir |
| SR 1334 Barwick Station Rd / SR 1330 Harold Sutton | 34 | 13 hour TMC | April 16, 2015 | Lenoir |
| SR 1690 Willie Measley Rd / SR 1603 Field Station Rd | 35 | 13 hour TMC | April 15, 2015 | Lenoir |
| SR 1548 Hill Farm Rd / SR 2021 Parrot Dickerson Rd | 36 | 13 hour TMC | April 21, 2015 | Lenoir |
| NC 11/55 at SR 1341 Tyree Rd | 40 | 13 hour TMC | April 24, 2015 | Lenoir |
| NC 11 at SR 1161 Green Haynes Rd / SR 1161 Albritton Rd | 41 | 13 hour TMC | May 5, 2015 | Lenoir |
| US 258 at SR 1900 Collier Loftin Rd/ Ferguson Ln | 45 | 13 hour TMC | April 28, 2015 | Lenoir |
| NC 58 at SR 1342 Will Baker Rd | 51 | 13 hour TMC | May 6, 2015 | Lenoir |
| SR 1821 British Rd at SR 1309 Caswell Rd | 54 | 13 hour TMC | May 8, 2015 | Jones |
| SR 1548 Hill Farm Rd at SR 1546 Banks School Rd | 62 | 13 hour TMC | April 22, 2015 | Lenoir |
| US 70 EB Left -U- East Sanderson | 66B | 13 hour TMC* | April 16, 2015 | Lenoir |
| US 258 at SR 1546 Banks School Rd | 67 | 13 hour TMC | April 23, 2015 | Lenoir |
| US 70 at SR 1552 Hillcrest Rd | 92 | 13 hour TMC | April 23, 2015 | Lenoir |
| US 258 at Road A | 98 | 13 hour TMC | April 24, 2015 | Lenoir |

[^0]B2. HISTORIC OVERVIEW TABLE: 1994-2014

| NCDOT CVRG | LOCATION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Project Count | Base Yr Forecast |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| VLM ID |  | 1994 | 1995 | 1996 | 1997 | 1998 | 1999 | 2000 | 2001 | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 | 2008 | 2009 | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2015 |
| $a$ | US 70 - East to West |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5300047 | US 70 West of NC 903 | 12,000 | 13,900 | 13,900 | 13,000 | 14,000 | 16,000 | 19,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 17,000 | 17,000 | 18,000 | 16,000 |  | 16,000 | 16,600 |
| 5300092 | US 70 West of Washington Street | 12,100 | 13,500 | 14,300 | 12,100 | 14,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 16,000 | 17,000 | 16,000 | 16,000 | 16,000 | 16,000 | 17,000 | 17,000 | 18,000 | 17,000 | 18,000 | 19,400 | 16,800 |
| 5300089 | US 70 East of Jim Sutton Road / Measley Road | 15,600 | 16,700 | 16,600 | 17,100 | 17,000 | 19,000 | 17,000 | 18,000 | 18,000 | 19,000 | 18,000 | 20,000 | 19,000 | 19,000 | 20,000 | 18,000 | 19,000 | 19,000 | 23,000 | 19,000 |  | 20,600 | 19,700 |
| 5300048 | US 70 East of Albert Sugg Road | 13,600 | 15,200 | 16,000 | 16,900 | 17,000 | 18,000 | 18,000 | 18,000 | 20,000 | 18,000 | 19,000 | 19,000 | 18,000 | 18,000 | 19,000 | 17,000 | 18,000 | 18,000 | 20,000 | 18,000 | 19,000 |  | 20,100 |
| 5300049 | US 70 West of Hill Farm Road |  |  | 18,600 | 19,300 |  | 20,000 | 20,000 | 21,000 |  | 23,000 | 21,000 | 23,000 | 22,000 | 22,000 | 23,000 | 20,000 | 22,000 | 23,000 | 24,000 | 23,000 |  | 27,900 | 25,400 |
| 5300034 | US 70 West of US 258 | 24,000 | 30,300 | 23,900 |  | 21,000 | 26,000 | 30,000 | 27,000 | 26,000 | 28,000 | 27,000 | 29,000 | 29,000 | 28,000 | 30,000 | 27,000 | 28,000 | 30,000 | 31,000 | 30,000 | 33,000 | 37,000 | 32,600 |
| 5300033 | US 70-258 East of US 258 | 33,800 | 35,700 | 36,600 | 37,700 | 37,000 | 39,000 | 39,000 | 37,000 | 41,000 | 41,000 | 44,000 | 43,000 | 39,000 | 40,000 | 44,000 | 37,000 | 39,000 | 43,000 | 43,000 | 39,000 |  | 44,200 | 39,600 |
| 5300029 | US 70-258 Business East of US 70-258 Bypass | 19,700 | 21,300 | 21,100 | 19,700 | 22,000 | 23,000 | 27,000 | 22,000 | 23,000 | 22,000 | 23,000 | 21,000 | 24,000 | 22,000 | 20,000 | 20,000 | 19,000 | 18,000 | 22,000 | 21,000 |  | 19,000 | 19,800 |
| 5300030 | US 70-258 BYP, East of US 70-258 BUS | 23,200 | 36,300 |  | 23,000 | 28,000 | 28,000 | 29,000 | 27,000 |  |  | 29,000 |  | 27,000 |  | 29,000 |  | 26,000 |  | 28,000 |  | 27,000 | 28,700 | 29,000 |
| 5300076 | US 70-258 Bypass West of NC 11 | 25,300 | 27,300 | 27,400 | 27,000 | 26,000 | 29,000 | 29,000 | 28,000 | 29,000 | 20,000 | 29,000 | 30,000 | 28,000 | 29,000 | 29,000 | 26,000 | 27,000 | 27,000 | 29,000 | 28,000 | 27,000 | 27,900 | 29,000 |
| 5300027 | US 70-258 Bypass East of NC 11 | 15,600 | 18,000 | 17,000 | 16,400 | 17,000 | 20,000 | 22,000 | 19,000 | 21,000 | 21,000 | 20,000 | 18,000 | 18,000 | 21,000 | 17,000 | 18,000 | 19,000 | 16,000 | 18,000 | 19,000 |  | 19,400 | 19,000 |
| 5300018 | US 70/NC 58, East of US 258 | 28,300 | 29,800 | 29,500 | 30,100 | 29,000 | 31,000 | 28,000 | 28,000 | 29,000 | 30,000 | 28,000 | 32,000 | 28,000 | 28,000 | 30,000 | 26,000 | 27,000 | 29,000 | 25,000 | 27,000 | 26,000 | 28,200 | 26,600 |
| 5300020 | US 70-258 Bypass West of NC 58 | 22,300 | 25,000 | 26,000 | 27,100 | 26,000 | 27,000 | 26,000 | 25,000 | 27,000 | 28,000 |  | 30,000 | 26,000 | 26,000 | 27,000 | 25,000 | 25,000 | 27,000 | 23,000 | 25,000 | 24,000 | 26,200 | 25,600 |
| 5300021 | US 70-258 Bypass East of NC 58 | 14,400 | 15,500 | 17,500 | 18,800 | 17,000 | 19,000 | 18,000 | 17,000 | 18,000 | 18,000 | 20,000 | 20,000 | 17,000 | 18,000 | 18,000 | 17,000 | 17,000 | 18,000 | 15,000 | 16,000 |  | 16,600 | 16,400 |
| 5300050 | US 70-258 Bypass West of Whaley Road |  |  |  | 13,100 |  | 14,000 | 15,000 | 14,000 | 15,000 | 14,000 | 14,000 | 15,000 | 13,000 | 15,000 | 14,000 | 14,000 | 13,000 | 14,000 | 13,000 | 14,000 | 13,000 | 15,000 | 14,800 |
| 5100001 | US 70 West of Wyse Fork Road |  |  |  |  |  | 12,000 | 13,000 | 13,000 | 13,000 | 13,000 | 14,000 | 13,000 | 12,000 | 13,000 | 13,000 | 12,000 | 13,000 | 12,000 | 12,000 | 12,000 |  | 14,000 | 13,600 |
| 5100032 | US 70 East of Wyse Fork Road |  |  |  |  |  | 10,000 | 12,000 | 11,000 | 11,000 | 11,000 | 11,000 | 12,000 | 10,000 | 12,000 | 12,000 | 11,000 | 12,000 | 11,000 | 11,000 | 10,000 | 10,000 | 13,300 | 12,800 |
| 5100031 | US 70 East of Burkett Road |  |  |  |  |  | 11,000 | 12,000 | 11,000 | 9,600 | 11,000 | 9,600 | 11,000 | 9,500 |  | 10,000 | 11,000 | 9,900 | 11,000 | 11,000 | 11,000 |  | 11,000 | 12,200 |
| b | US 258 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5300083 | US 258, N of SR 1546 (Banks School Rd) | 11,900 | 14,500 | 11,300 | 9,300 |  | 12,000 | 13,000 | 13,000 | 14,000 | 15,000 | 15,000 | 17,000 | 15,000 | 15,000 | 16,000 | 14,000 | 14,000 | 16,000 | 14,000 | 15,000 |  | 14,200 | 14,000 |
| 5300032 | US 258 North of US 70 | 11,600 | 13,500 | 11,800 | 14,900 | 11,000 | 11,000 | 13,000 | 12,000 | 15,000 | 13,000 | 13,000 | 15,000 | 13,000 | 13,000 | 13,000 | 11,000 | 11,000 | 11,000 | 12,000 | 11,000 | 11,000 | 12,800 | 11,800 |
| 5300019 | US 258, South of US 70-258 BYP | 12,800 | 12,900 | 13,000 | 12,000 | 12,000 | 12,000 | 11,000 | 11,000 | 10,000 | 11,000 | 10,000 | 12,000 | 10,000 | 10,000 | 11,000 | 9,300 | 10,000 | 11,000 | 9,500 |  |  | 11,200 | 10,600 |
| 5300079 | US 258, North of SR 1900 (Collier Loftin Rd) | 9,400 | 10,500 | 10,300 | 9,200 | 10,000 | 12,000 | 10,000 | 9,600 | 9,600 | 10,000 | 9,500 | 10,000 | 9,300 | 9,200 | 9,700 | 9,500 | 9,100 | 10,000 | 8,500 | 8,700 |  | 10,300 | 10,100 |
| 5300078 | US 258, South of SR 1900 (Collier Loftin Rd) | 9,000 | 10,600 | 9,800 | 8,900 | 9,800 | 11,000 | 9,400 | 9,600 | 9,500 | 10,000 | 9,600 | 9,700 | 9,100 | 9,100 | 9,600 | 9,500 | 9,600 | 10,000 | 8,500 | 8,600 | 8,700 | 10,100 | 10,000 |
| 5300035 | US 258, North of SR 1342 (Will Baker Rd) | 7,700 | 8,100 | 8,400 | 8,000 | 8,500 | 9,300 | 9,000 | 99,000 | 8,500 | 7,000 | 8,300 | 9,100 | 8,600 | 8,500 | 8,900 | 9,200 | 9,300 | 8,700 | 8,200 | 8,000 |  |  | 10,000 |
| 5300036 | US 258, South of SR 1341 (Tyree Rd) | 7,400 | 7,900 | 8,300 | 8,400 | 8,500 | 9,200 | 8,400 | 8,500 | 8,700 | 8,700 | 7,800 | 8,600 | 7,900 | 8,200 | 8,500 | 8,700 | 8,000 | 8,500 | 8,000 | 8,000 | 8,400 |  | 7,400 |
| 5300099 | US 258, South of SR 1161 (Albritton Rd) | 5,900 | 6,700 | 7,200 | 7,100 | 7,200 | 7,600 |  | 7,100 | 7,100 | 7,600 | 6,600 | 6,900 | 6,900 | 6,600 | 7,600 | 7,700 | 7,100 | 7,000 | 6,900 | 6,700 |  |  | 7,000 |
| c | NC 11 / NC 55 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5300026 | NC 11/ NC 55 North of US 70 | 12,200 | 14,100 | 12,100 | 13,900 | 12,000 | 13,000 | 13,000 | 13,000 | 14,000 | 17,000 | 13,000 | 13,000 | 12,000 | 13,000 | 12,000 | 12,000 | 12,000 | 11,000 | 13,000 | 13,000 | 11,000 | 12,400 | 13,000 |
| 5300028 | NC 11-55, South of US 70-258 BYP | 17,600 | 18,600 | 18,600 | 16,400 | 17,000 | 18,000 | 19,000 | 18,000 | 20,000 |  | 19,000 | 18,000 | 17,000 | 19,000 |  | 17,000 |  | 16,000 | 17,000 | 17,000 | 16,000 | 16,600 | 17,000 |
| 5300037 | NC 11/ NC 55 North of NC 11 and NC 55 Merge | 11,000 | 12,000 | 12,100 | 12,000 | 13,000 | 14,000 |  | 13,000 | 14,000 | 14,000 | 14,000 | 15,000 | 15,000 | 14,000 | 15,000 | 14,000 |  | 14,000 | 15,000 | 14,000 |  | 12,500 | 12,600 |
| 5300039 | NC 11 South of NC 11 / NC 55 Merge | 8,200 | 9,400 | 9,200 | 8,600 | 8,800 | 9,500 |  |  | 10,000 | 11,000 | 10,000 | 11,000 | 11,000 | 10,000 | 11,000 | 11,000 | 9,800 | 12,000 | 11,000 | 10,000 |  | 9,600 | 10,400 |
| 5300068 | NC 11, North of SR 1161 (Albritton Rd) | 7,200 | 7,300 | 8,300 | 7,300 | 8,100 | 8,400 |  |  | 9,400 | 10,000 | 9,700 | 11,000 | 10,000 | 9,800 | 11,000 | 9,600 | 9,000 | 10,000 | 10,000 | 9,500 | 9,700 | 10,000 | 10,400 |
| 5300067 | NC 11, South of SR 1161 (Albritton Rd) | 6,800 | 7,100 | 8,100 | 8,200 | 7,800 | 8,400 |  |  | 9,200 | 10,000 | 9,000 | 10,000 | 10,000 | 9,400 | 10,000 | 9,300 | 8,800 | 9,900 | 9,800 | 10,000 |  | 9,600 | 10,000 |
| 5300038 | NC 55 South of NC 11 / NC 55 Merge | 4,300 | 4,100 | 4,400 | 4,800 | 4,700 | 5,600 |  | 3,600 | 4,900 | 5,000 | 4,700 |  | 5,100 | 4,700 | 5,100 | 5,100 | 4,800 | 4,900 | 5,700 | 4,600 | 4,800 | 4,400 | 4,800 |
| d | NC 58 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5300017 | US 70-258 BUS/NC 58, North of US 70-258 BYP |  |  |  |  |  |  |  |  | 16,000 | 17,000 | 16,000 | 16,000 | 15,000 | 16,000 | 14,000 | 14,000 | 14,000 | 14,000 | 14,000 | 13,000 |  | 16,100 | 15,000 |
| 5300022 | NC 58, South of US 70 | 8,800 | 10,600 | 9,500 | 9,000 | 10,000 | 8,000 | 7,800 | 7,500 | 8,100 | 8,500 | 9,700 | 12,000 | 10,000 | 9,400 | 11,000 | 9,100 | 9,300 | 11,000 | 9,000 | 10,000 | 11,000 | 11,200 | 11,400 |
| 5300053 | NC 58, North of SR 1913 (Elijah Loftin Rd) | 5,100 | 6,200 | 5,200 | 5,700 | 5,900 | 5,900 | 5,800 | 5,700 | 6,000 | 5,900 | 6,500 | 5,800 | 5,000 | 5,600 | 6,000 | 5,300 | 5,100 | 5,500 | 5,200 | 5,100 |  |  | 5,600 |
| 5300091 | NC 58, South of SR 1913 (Elijah Loftin Rd) | 4,300 | 4,500 | 4,300 | 5,500 | 4,600 | 4,900 | 4,900 |  | 5,000 | 4,900 | 5,400 | 4,800 | 4,700 | 4,600 | 4,900 | 4,100 | 4,300 | 5,500 | 4,200 | 5,200 |  | - | 4,900 |
| e | NC 903 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5300104 | NC 903, North of SR 1378 (Packhouse Rd) | 2,800 | 2,800 | 3,100 | 2,500 | 3,100 | 6,100 |  | 3,400 | 3,300 | 3,500 | 3,500 | 4,000 | 3,700 | 3,600 | 3,800 | 3,500 | 3,200 | 3,800 | 3,700 | 3,700 | 3,400 | 4,000 | 4,000 |
| 5309018 | NC 903, South of US 70 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 3,900 |  |  | 4,000 |  | 4,400 | 4,100 | 4,000 |

B2. HISTORIC OVERVIEW TABLE: 1994-2014

| NCDOT CVRG | LOCATION |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Project Count | Base Yr Forecast |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| VLM ID |  | 1994 | 1995 | 1996 | 1997 | 1998 | 1999 | 2000 | 2001 | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 | 2008 | 2009 | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2015 |
| $f$ | Local Roads Intersecting US 70 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5301534 | SR 1603 (Washington St), North of US 70 |  | 4,300 |  | 4,000 |  | 4,500 |  |  |  | 4,300 |  | 4,400 |  | 3,800 |  | 4,000 |  | 4,100 |  | 3,900 |  | 4,600 | 4,000 |
| 5301532 | SR 1327 (Jim Sutton Rd), South of US 70 |  | 800 |  | 680 |  | 760 |  | 900 |  | 1,000 |  | 1,000 |  | 880 |  | 1,000 |  | 970 |  |  |  | 1,500 | 1,400 |
| 5303416 | SR 1520 ( (Norbert Hill Rd), North of US 70 | 400 |  | 560 |  | 700 |  | 560 |  |  | 510 |  | 490 |  |  |  | 400 |  | 540 |  | 470 |  | - | 600 |
| 5301556 | SR 1522 (Albert Sugg Rd), North of US 70 |  | 280 |  | 230 |  | 260 |  | 260 | 190 |  | 210 |  | 330 |  | 230 |  | 220 |  | 270 |  | 200 | - | 400 |
| 5301560 | SR 1330 (Harold Sutton Rd), South of US 70 |  | 650 |  | 600 |  | 590 |  | 650 |  | 610 |  | 700 |  |  |  | 620 |  | 650 |  | 750 |  | - | 400 |
| 5301555 | SR 1519 (Eason Rd), North of US 70 |  | 1,000 |  | 1,000 |  | 1,100 |  | 1,000 |  | 1,000 |  | 930 |  | 1,000 |  | 880 |  | 1,000 |  | 940 |  | 1,100 | 1,100 |
| 5301558 | SR 1324 (Kennedy Home Rd), South of US 70 |  | 1,600 |  | 1,600 |  | 1,700 |  | 1,900 |  | 1,700 |  | 2,000 |  | 2,000 |  | 1,800 |  | 1,700 |  | 1,800 |  | 1,700 | 1,700 |
| 5301557 | SR 1546 (Banks School Rd), North of US 70 |  | 1,300 |  | 1,400 |  | 1,700 |  | 1,800 |  | 2,000 |  | 2,000 |  | 1,900 |  | 1,500 |  | 1,600 |  | 1,500 |  | 1,200 | 1,500 |
| 5301744 | SR 1804 (Neuse Rd), North of US 70 |  | 2,200 |  | 2,400 |  | 2,900 |  | 2,200 |  | 2,400 |  | 2,300 |  | 2,000 |  | 2,100 |  | 2,200 |  |  |  | 1,700 | 1,800 |
| 5301743 | SR 1903 (Cobb Rd), South of US 70 |  | 1,500 |  | 1,700 |  | 1,800 |  | 1,700 | 1,600 |  | 1,700 |  | 1,700 |  | 1,800 |  | 1,500 |  | 1,500 |  |  | 1,900 | 1,800 |
| 5301618 | SR 1821 (British Rd), North of US 70 |  |  |  |  |  |  |  |  |  | 800 |  | 780 |  | 740 |  | 810 |  | 710 |  |  |  | 600 | 600 |
| 5101502 | SR 1309 (Caswell Station Rd), North of US 70 |  |  |  |  |  |  |  |  | 580 |  | 490 |  | 500 |  | 410 |  | 380 |  | 450 |  | 300 | 500 | 400 |
| 5101504 | SR 1002 (Wyse Fork Rd), South of US 70 |  |  |  |  |  | 1,500 |  | 1,500 |  | 1,400 |  | 1,500 |  | 1,500 |  | 1,300 |  | 1,200 |  | 1,300 |  | 1,700 | 1,600 |
| 5101503 | SR 1312 (Tilghman Rd), North of US 70 |  |  |  |  |  |  |  |  |  | 520 |  | 400 |  | 440 |  | 380 |  | 350 |  | 340 |  | - | 400 |
| 5101562 | SR 1005 (Old US Hwy 70), North of US 70 |  |  |  |  |  |  |  |  |  | 2,100 |  | 2,400 |  | 2,200 |  | 2,400 |  | 2,200 |  | 2,100 |  | 1,600 | 1,700 |
| 9 | Other Local Roads |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5303414 | SR 1309 (Jenny Lind Rd), South of NC 903 |  |  |  |  |  |  |  |  | 1,700 |  | 1,500 |  | 1,900 |  | 1,700 |  | 1,500 |  | 1,600 |  | 1,600 | 1,600 | 1,800 |
| 5301533 | SR 1690 (Willie Measley Rd), South of SR 1503 |  |  |  | 1,900 |  | 2,200 |  | 2,500 | 2,100 |  | 1,900 |  | 2,300 |  | 2,900 |  | 2,400 |  | 2,700 |  |  | 2,000 | 2,100 |
| 5301679 | SR 1548 (Hill Farm Rd), North of SR 2021 |  |  |  | 3,600 |  | 4,600 |  | 3,500 |  | 4,800 |  | 5,100 |  | 5,000 |  | 6,000 |  | 5,900 |  | 5,400 |  | 4,800 | 4,800 |
| 5301742 | SR 1903 (Cobb Rd), South of SR 1904 |  | 950 |  | 1,000 |  | 1,100 |  | 1,100 |  | 1,000 |  | 1,000 |  | 960 |  | 1,100 |  | 940 |  | 850 |  | 600 | 600 |
| 5101501 | SR 1310 (British Rd), East of SR 1309 |  |  |  |  |  |  |  |  |  | 740 |  | 710 |  | 810 |  | 770 |  | 680 |  | 1,200 |  | 600 | 600 |
| 5101500 | SR 1309, North of SR 1310 (British Rd) |  |  |  |  |  |  |  |  | 370 |  | 310 |  | 360 |  | 230 |  | 290 |  | 290 |  | 230 | 200 | 400 |
| 5301731 | SR 1351 (Old Asphalt Rd), East of NC 11-55 |  | 1,900 |  | 1,700 |  | 1,800 |  | 1,800 | 690 |  | 880 |  | 870 |  | 1,000 |  | 710 |  | 670 |  |  | - | 1,700 |
| 5301730 | SR 1351 (Old Asphalt Rd) , West of US 258 |  | 1,400 |  | 1,500 |  | 1,400 |  | 920 |  | 590 |  | 560 |  | 680 |  | 520 |  | 410 |  | 380 |  | - | 1,700 |
| 5301678 | SR 1546 (Banks School Rd), West of US 258 |  |  |  | 5,900 |  | 5,600 |  | 6,700 | 6,900 |  | 7,400 |  | 7,700 |  | 8,800 |  | 7,600 |  | 8,700 |  | 8,700 | 8,300 | 8,200 |
| 5301728 | SR 1357 (Meadowbrook Dr), West of NC 58 |  | 1,000 |  | 1,000 |  | 1,300 |  | 930 |  | 770 |  | 1,000 |  | 940 |  | 880 |  | 910 |  | 850 |  | - | 1,800 |
| 5301677 | SR 1342 (Central Ave), East of NC 11-55 |  | 230 |  | 300 |  | 400 |  | 200 | 340 |  | 290 |  | 350 |  | 370 |  | 340 |  | 340 |  |  | - | 400 |
| 5301739 | SR 1342 (Will Baker Rd), West of NC 58 |  | 1,400 |  | 1,600 |  | 1,900 |  | 1,900 |  | 1,800 |  | 1,800 |  | 1,500 |  | 1,500 |  | 1,700 |  |  |  | 1,500 | 1,500 |
| 5301735 | SR 1341 (Tyree Rd), East of NC 11-55 |  | 2,200 |  | 2,400 |  | 2,800 |  | 2,400 |  | 2,800 |  | 2,900 |  | 2,800 |  | 2,800 |  | 3,000 |  | 2,400 |  | 2,600 | 3,000 |
| 5301564 | SR 1161 (Green Haynes Rd), East of NC 55 |  | 600 |  | 750 |  | 1,000 |  | 800 |  | 880 |  | 800 |  | 770 |  | 800 |  | 760 |  | 760 |  | - | 800 |
| 5303466 | SR 1161 (Green Haynes Rd), West of NC 11 |  |  |  |  |  |  |  |  |  | 2,000 |  | 1,900 |  | 1,900 |  | 1,900 |  | 1,800 |  | 1,900 |  | 1,900 | 2,000 |
| 5301669 | SR 1161 (Albritton Rd), East of NC 11 |  | 900 |  | 1,100 |  | 1,300 |  |  | 1,200 |  | 1,200 |  | 1,400 |  | 1,300 |  | 1,200 |  | 1,300 |  |  | 1,200 | 1,200 |
| 5301603 | SR 1161 (Albritton Rd), West of US 258 |  | 440 |  | 420 |  | 500 |  | 500 | 530 |  | 470 |  | 670 |  | 630 |  | 550 |  | 590 |  |  | - | 1,200 |
| 5303460 | SR 1149 (Joe Nunn Rd), West of US 258 |  |  |  |  |  |  |  |  |  | 1,300 |  | 1,400 |  | 1,100 |  | 1,200 |  | 1,200 |  | 970 |  | - | - |
| 5301562 | SR 1157 (N Croom Bland Rd), North of NC 55 |  | 70 |  | 60 |  | 50 |  | 50 | 40 |  | 80 |  | 50 |  |  |  | 40 |  | 50 |  | 60 | - | 100 |
| 5301741 | SR 1909 (Woodington Rd), West of NC 58 |  |  |  | 950 |  | 1,000 |  | 940 |  | 860 |  | 840 |  |  |  | 810 |  | 880 |  | 790 |  | - | 800 |
| 5301737 | SR 1900 (Collier Loftin Rd), East of US 258 |  | 1,600 |  | 1,300 |  | 2,400 |  | 1,400 | 1,500 |  | 1,500 |  | 1,900 |  | 1,900 |  | 1,300 |  | 1,200 |  |  | 1,700 | 1,700 |
| 5301738 | SR 1900 (Collier Loftin Rd), West of NC 58 |  | 1,000 |  | 900 |  | 1,600 |  | 990 | 1,200 |  | 1,400 |  | 1,600 |  | 1,300 |  | 1,000 |  | 860 |  | 1,300 | - | 1,700 |
| 5301616 | SR 1913 (Elijah Loftin Rd), East of NC 58 |  | 640 |  | 850 |  | 1,200 |  | 940 | 1,100 |  | 890 |  | 1,000 |  | 1,100 |  | 830 |  | 850 |  | 820 | - | 700 |
| 5301615 | SR 1913 (Elijah Loftin Rd), West of NC 58 |  | 1,100 |  | 850 |  | 1,200 |  | 1,000 | 1,100 |  | 1,000 |  | 1,200 |  | 1,300 |  | 850 |  | 930 |  |  | - | 800 |
| 2401686 | SR 1005, East of SR 1268 (Carmichael Ave) |  |  |  |  |  |  |  |  | 2,000 |  | 1,900 |  | 1,800 |  |  |  | 1,900 |  | 1,800 |  | 1,700 | - | 1,400 |
| 2401639 | SR 1262 (N West St), North of SR 1005 |  |  |  |  |  |  |  |  |  | 1,600 |  | 1,600 |  | 1,400 |  | 1,300 |  | 1,300 |  | 1,200 |  | - | 800 |
| 2401638 | SR 1262 (N West St), North of SR 1270 |  |  |  |  |  |  |  |  |  | 280 |  | 440 |  | 200 |  | 250 |  | 130 |  | 130 |  | - | 200 |

2015 Project Specific Turn Movement Counts factored to AADT estimates
While comparing a row with NCDOT Historical AADT, note that count and forecasts are in close proximity but not at exact location of NCDOT's count station

B3. GROWTH RATE COMPARISON - Historic, Model and Forecast

| KEY LOGATIONS | Annualized Growth Rate based on Historic AADT Counts |  |  | Annualized Growth Rate based on Travel Demand Model Volumes |  |  | Annualized Growth Rate Utilized in No Build Forecast |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1994-2004 (10 Yr) | 2004-2014 (10 Yr) | 1994 to 2004 (20 yr) | 2011 | 2040 | 2011 to 2040 | 2015 | 2040 | 2015 to 2040 |
| US 70 - East to West |  |  |  |  |  |  |  |  |  |
| US 70 West of NC 903 | 2.9\% |  |  | 17,500 | 35,300 | 2.4\% | 16,600 | 35,400 | 3.1\% |
| US 70 West of Washington Street | 2.8\% | 1.2\% | 2.0\% | 19,500 | 37,300 | 2.3\% | 16,800 | 35,600 | 3.0\% |
| US 70 East of Jim Sutton Road / Measley Road | 1.4\% |  |  | 21,400 | 38,900 | 2.1\% | 19,700 | 38,200 | 2.7\% |
| US 70 East of Albert Sugg Road | 3.4\% | 0.0\% | 1.7\% | 21,500 | 39,000 | 2.1\% | 20,100 | 38,600 | 2.6\% |
| US 70 West of Hill Farm Road | 1.6\% |  |  | 17,100 | 28,900 | 1.8\% | 25,400 | 36,200 | 1.4\% |
| US 70 West of US 258 | 1.2\% | 2.0\% | 1.6\% | 28,100 | 39,100 | 1.1\% | 32,600 | 43,000 | 1.1\% |
| US 70-258 East of US 258 | 2.7\% |  |  | 33,300 | 45,000 | 1.0\% | 39,600 | 49,000 | 0.9\% |
| US 70-258 Business East of US 70-258 Bypass | 1.6\% |  |  | 18,800 | 20,700 | 0.3\% | 19,800 | 22,400 | 0.5\% |
| US 70-258 BYP, East of US 70-258 BUS | 2.3\% | -0.7\% | 0.8\% | 25,700 | 37,500 | 1.3\% | 29,000 | 40,200 | 1.3\% |
| US 70-258 Bypass West of NC 11 | 1.4\% | -0.7\% | 0.3\% | 25,800 | 37,600 | 1.3\% | 29,000 | 40,200 | 1.3\% |
| US 70-258 Bypass East of NC 11 | 2.5\% |  |  | 17,600 | 27,700 | 1.6\% | 19,000 | 30,400 | 1.9\% |
| US 70/NC 58, East of US 258 | -0.1\% | -0.7\% | -0.4\% | 20,100 | 32,900 | 1.7\% | 26,600 | 37,200 | 1.4\% |
| US 70-258 Bypass West of NC 58 | 2.7\% | -1.9\% | 0.4\% | 20,100 | 32,900 | 1.7\% | 25,600 | 36,200 | 1.4\% |
| US 70-258 Bypass East of NC 58 | 3.3\% |  |  | 15,500 | 28,300 | 2.1\% | 16,400 | 29,400 | 2.4\% |
| US 70-258 Bypass West of Whaley Road |  | -0.7\% | 0.2\% | 14,600 | 27,400 | 2.2\% | 14,800 | 27,600 | 2.5\% |
| US 70 West of Wyse Fork Road |  |  |  | 14,100 | 27,100 | 2.3\% | 13,600 | 26,400 | 2.7\% |
| US 70 East of Wyse Fork Road |  | -0.9\% |  | 12,900 | 25,900 | 2.4\% | 12,800 | 25,400 | 2.8\% |
| US 70 East of Burkett Road |  |  |  | 13,500 | 27,300 | 2.5\% | 12,200 | 24,400 | 2.8\% |
| US 258 |  |  |  |  |  |  |  |  |  |
| US 258, N of SR 1546 (Banks School Rd) | 2.3\% |  |  | 11,500 | 13,300 | 0.5\% | 14,000 | 15,800 | 0.5\% |
| US 258 North of US 70 | 1.1\% | -1.7\% | -0.3\% | 10,100 | 11,500 | 0.4\% | 11,800 | 13,200 | 0.4\% |
| US 258, South of US 70-258 BYP | -2.4\% |  |  | 9,400 | 10,600 | 0.4\% | 10,600 | 13,000 | 0.8\% |
| US 258, North of SR 1900 (Collier Loftin Rd) | 0.1\% |  |  | 6,500 | 7,600 | 0.5\% | 10,100 | 12,400 | 0.8\% |
| US 258, South of SR 1900 (Collier Loftin Rd) | 0.6\% | -1.0\% | -0.2\% | 6,100 | 7,100 | 0.5\% | 10,000 | 12,300 | 0.8\% |
| US 258, North of SR 1342 (Will Baker Rd) | 0.8\% |  |  | 5,800 | 6,800 | 0.6\% | 10,000 | 12,300 | 0.8\% |
| US 258, South of SR 1341 (Tyree Rd) | 0.5\% | 0.7\% | 0.6\% | 6,800 | 8,000 | 0.6\% | 7,400 | 9,200 | 0.9\% |
| US 258, South of SR 1161 (Albritton Rd) | 1.1\% |  |  | 7,400 | 8,600 | 0.5\% | 7,000 | 8,800 | 0.9\% |
| NC 11 / NC 55 |  |  |  |  |  |  |  |  |  |
| NC 11/ NC 55 North of US 70 | 0.6\% | -1.7\% | -0.5\% | 10,600 | 11,300 | 0.2\% | 13,000 | 14,200 | 0.4\% |
| NC 11-55, South of US 70-258 BYP | 0.8\% | -1.7\% | -0.5\% | 19,700 | 22,600 | 0.5\% | 17,000 | 22,000 | 1.0\% |
| NC 11/ NC 55 North of NC 11 and NC 55 Merge | 2.4\% |  |  | 14,600 | 17,500 | 0.6\% | 12,600 | 17,900 | 1.4\% |
| NC 11 South of NC 11 / NC 55 Merge | 2.0\% |  |  | 10,400 | 13,100 | 0.8\% | 10,400 | 15,500 | 1.6\% |
| NC 11, North of SR 1161 (Albritton Rd) | 3.0\% | 0.0\% | 1.5\% | 10,000 | 12,000 | 0.6\% | 10,400 | 15,500 | 1.6\% |
| NC 11, South of SR 1161 (Albritton Rd) | 2.8\% |  |  | 9,100 | 12,000 | 1.0\% | 10,000 | 15,000 | 1.6\% |
| NC 55 South of NC 11 / NC 55 Merge | 0.9\% | 0.2\% | 0.6\% | 5,100 | 5,500 | 0.3\% | 4,800 | 5,200 | 0.3\% |
| NC 58 |  |  |  |  |  |  |  |  |  |
| US 70-258 BUS/NC 58, North of US 70-258 BYP |  |  |  | 13,100 | 16,700 | 0.8\% | 15,000 | 18,800 | 0.9\% |
| NC 58, South of US 70 | 1.0\% | 1.3\% | 1.1\% | 6,900 | 10,600 | 1.5\% | 11,400 | 12,400 | 0.3\% |
| NC 58, North of SR 1913 (Elijah Loftin Rd) | 2.5\% |  |  | 5,600 | 5,800 | 0.1\% | 5,600 | 6,200 | 0.4\% |
| NC 58, South of SR 1913 (Elijah Loftin Rd) | 2.3\% |  |  | 4,500 | 4,600 | 0.1\% | 4,900 | 5,400 | 0.4\% |
| NC 903 |  |  |  |  |  |  |  |  |  |
| NC 903, North of SR 1378 (Packhouse Rd) | 2.3\% | -0.3\% | 1.0\% | 4,400 | 4,800 | 0.3\% | 4,000 | 4,800 | 0.7\% |
| NC 903, South of US 70 |  |  |  | 5,700 | 6,300 | 0.3\% | 4,000 | 5,600 | 1.4\% |

## B4. COMPARISON WITH PREVIOUS FORECASTS

| KEY LOCATIONS | R-2544 Goldsboro Bypass |  | R-2719A Crescent Road June 2004 |  | NC 11-NC 55 Bypass Feasibility Study (FS-0802A) |  | kinston Area Comprehensive Transportation Plan (not fiscally |  | R-2553 Kinston Bypass July 2009 |  |  | R-2553 Kinston Bypass 2012 update |  |  | R-2553 Kinston Bypass 2015 update |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1998 | 2018 | 2004 | 2030 | 2008 | Build (includes R 2553 | 2008 | Existing Network (with CF | 2008 | 2020 | 2035 | 2012 | 2020 | 2040 | 2015 | 2040 |
| US 70 - West to East |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 70 West of NC 903 | 32,600 | 59,200 | n/a | n/a | n/a | n/a | 18,000 | 65,500 | 16,000 | 37,800 | 55,200 | 16,100 | 25,000 | 36,000 | 16,600 | 35,400 |
| US 70 East of Washington Street | n/a | NA | 22,500 | 43,400 | n/a | n/a | 22,000 | 64,100 | 20,000 | 43000 | 61,400 | 19,600 | 32,000 | 41,800 | 20,200 | 39,200 |
| US 70 West of C.F. Harvey Pkwy | n/a | NA | 34,200 | 32,600 | n/a | n/a | 28,000 | 64,100 | 32,000 | 40,400 | 79,200 | 18,400 | 29,000 | 37,000 | 21,200 | 39,000 |
| US 70 West of US 258 | n/a | NA | n/a | 32,600 | n/a | n/a | n/a | 32,400 | n/a | n/a | 52,800 | 31,600 | 35,000 | 46,000 | 32,600 | 43,000 |
| US 70-258 East of US 258 | n/a | NA | 42,800 | 35,100 | n/a | n/a | 40,000 | 57,100 | 40,000 | 50,600 | 62,600 | 30,000 | 42,000 | 54,600 | 39,600 | 49,000 |
| US 70-258 Bypass East of NC 11 | n/a | NA | n/a | n/a | 21,000 | 19,300 | 21,000 | 40,000 | 20,800 | 35,200 | 46,000 | 20,800 | 22,400 | 32,000 | 19,000 | 29,600 |
| US 70-258 Bypass West of NC 58 | n/a | NA | n/a | n/a | 28,700 | 27,200 | 27,000 | 51,100 | 27,000 | 42,300 | 54,600 | 27,000 | 28,800 | 38,200 | 25,600 | 35,000 |
| US 70 East of Wyse Fork Road | n/a | NA | n/a | n/a | n/a | n/a | 15,000 | 31,900 | 12,000 | 24,800 | 35,000 | 13,400 | 18,000 | 27,200 | 12,800 | 25,000 |
| CF Harvey Parkway |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| CF Harvey Parkway Extension (US 70 to US 258) | n/a | n/a | n/a | 20,400 | n/a | n/a | n/a | 45,600 | n/a | 27,600 | 43,400 | 3,000 | 4,400 | 6,400 | 2,800 | 9,400 |
| CF Harvey Parkway east of US 258 | n/a | n/a | 5,600 | 23,400 | n/a | n/a | n/a | 33,000 | 4,800 | 28,300 | 47,200 | 6,000 | 7,200 | 10,000 | n/a | n/a |
| CF Harvey Parkway west of NC 58 | n/a | n/a | n/a | n/a | n/a | n/a | n/a | 25,400 | 2,000 | 16,800 | 27,800 | 2,400 | 7,600 | 10,600 | n/a | n/a |
| Roads north of US 70 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| US 258 North of US 70 | n/a | n/a | 15,000 | 32,400 | n/a | n/a | 14,000 | 45,600 | 16,200 | 21,400 | 28,200 | 11,600 | 11,000 | 13,000 | 11,800 | 13,200 |
| US 70 Bus Vernon Avenue | n/a | n/a | n/a | n/a | n/a | n/a | 22,000 | 40,900 | 22,400 | 24,800 | 30,000 | 18,800 | 20,600 | 24,200 | 19,800 | 22,400 |
| NC 55/ NC 11 Old Pink Hill Road | n/a | n/a | n/a | n/a | 15,000 | 24,200 | 13,000 | 42,400 | 13,400 | 21,500 | 28,000 | 14,000 | 14,000 | 16,000 | 13,000 | 14,200 |
| US 258 /Queen Street | n/a | n/a | n/a | n/a | 17,600 | 26,200 | 16,000 | 57,200 | 16,200 | 31,200 | 44,000 | 16,000 | 17,200 | 19,200 | 15,000 | 18,800 |
| Roads south of US 70 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| NC 55/ NC 11 Old Pink Hill Road | n/a | n/a | n/a | n/a | 18,200 | 22,400 | 19,000 | 43,200 | 19,200 | 29,100 | 37,000 | 17,600 | 19,000 | 22,400 | 17,000 | 22,000 |
| US 258 | n/a | n/a | n/a | n/a | 11,000 | 20,200 | 10,000 | 39,600 | 10,600 | 25,500 | 37,400 | 10,200 | 11,000 | 13,400 | 10,600 | 13,000 |
| NC 58 | n/a | n/a | n/a | n/a | 10,700 | 8,800 | 9,400 | 22,400 | 10,400 | 15,700 | 20,000 | 11,400 | 13,000 | 14,600 | 11,400 | 12,400 |

Source
R-2554 Goldsboro Bypass - from public hearing map
R-2719AC.F. Harvey Parkway- June 2004
FS - O802ANC 11- N C 55 Bypass Feasibility Study - October 2010
Kinston Area Comprehensive Transportation Plan - February 2008
R-2553 Kinston Bypass- July 2009
R-2553 Kinston Bypass- July 2012

| Location | AADT | D - Directional Distribution |  |  | K - Peak Hour Factor |  |  | Heavy Vehicles - (Dual / TTST) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $2015$ <br> Class Count | 2012 <br> TIP Project <br> R-2553 <br> Forecast | $2015$ <br> Class Count | 2015 <br> Selected BY NB Value | 2012 TIP Project R-2553 Forecast | $2015$ <br> Class Count | $\begin{gathered} 2015 \\ \text { Selected BY } \\ \text { NB Value } \end{gathered}$ | 2012 <br> TIP Project <br> R-2553 <br> Forecast | $2015$ <br> Class Count | $\begin{gathered} 2015 \\ \text { Selected BY } \\ \text { NB Value } \end{gathered}$ |
| US 70 West of SR 1334 (Barwick Station Rd) | 20,600 | 60\% | 55\% | 55\% | 9\% | 8.0\% | 9\% | $5 / 10$ | 4.3 / 8.0 | 4 / 8 |
| US 70 West of Pinelawn Cemetery Dr | 22,400 | 55\% | 54\% | 50\% | 10\% | 8.1\% | 9\% | $5 / 8$ | $3.8 / 6.5$ | $4 / 6$ |
| US 70-US 258 Bypass West of US 70 Bus | 36,700 | 55\% | 51\% | 55\% | 9\% | 7.6\% | 9\% | 5/5 | 3.2 / 4.9 | 4 / 5 |
| US 70-US 258 Bypass East of US 70 Bus | 26,800 | 60\% | 53\% | 55\% | 9\% | 8.0\% | 9\% | $6 / 7$ | $3.6 / 5.2$ | $5 / 6$ |
| US 70 East of Lenoir CC | 14,800 | 55\% | 53\% | 55\% | 9\% | 7.6\% | 8\% | $5 / 8$ | $9.1 / 9.0$ | 6 / 9 |
| US 70 East of SR 1313 (Burkett Road) | 11,000 | 60\% | 50\% | 55\% | 9\% | 8.0\% | 8\% | 5/10 | 7.0 / 12.0 | 7 / 12 |
| C.F. Harvey Parkway North of US 70 | 2,800 | 55\% | 61\% | 60\% | 10\% | 9.7\% | 9\% | $8 / 6$ | 4.8 / 8.4 | 5/9 |
| NC 11/55 North of NC 11 and 55 Merge | 12,500 | 65\% | 61\% | 65\% | 9\% | 8.0\% | 10\% | $4 / 4$ | 5.6 / 8.1 | 6 / 7 |
| US 258 South of SR 1149 (Joe Nunn Road) | 7,100 | 60\% | 64\% | 60\% | 9\% | 8.8\% | 9\% | $5 / 6$ | $7.4 / 5.3$ | $8 / 5$ |
| NC 58 South of SR 1900 (Collier Loftin Road) | 7,000 | 65\% | 73\% | 65\% | 9\% | 8.9\% | 9\% | 3 / 3 | $3.2 / 4.2$ | 4 / 4 |
| SR 1324 (Kennedy Home Road) South of US 70 | 1,400 | - | 73\% | 60\% | - | 8.3\% | 9\% | - | 4.4 / 1.2 | $3 / 1$ |
| SR 1324 (Kennedy Home Road) South of SR 1336 (Barrus Road) | 1,500 | - | 78\% | 60\% | - | 8.7\% | 9\% | - | 3.1 / 0.8 | 3 / 1 |
| SR 2002 (Innovation Way) North of US 70 * | 2,400 | - | 78\% | 65\% | - | 10.8\% | 12\% | - | 3.0 / 7.9 | $3 / 8$ |
| SR 1913 (Elijah Loftin Road) East of SR 1904 (Whaley Road) | 600 | - | 77\% | 75\% | - | 12.1\% | 12\% | - | 3.2 / 0.4 | $3 / 1$ |
| SR 1903 (Cobb Road) North of SR 1913 (Elijah Loftin Road) | 600 | - | 73\% | 70\% | - | 8.8\% | 9\% | - | $4.7 / 0.6$ | $5 / 1$ |
| SR 1911 (Alexander Rouse Rd) West of SR 1912 (Parker Fork Rd) | 200 | - | 58\% | - | - | 8.4\% | - | - | 9.1 / 1.0 | $9 / 1$ |
| SR 1139 (Clarence Potter Road) West of US 258 | 100 | - | 88\% | 60\% | - | 29.1\% | 9\% | - | 1.1 / 0 | $1 / 1$ |
| Exit of Lenoir CC East of NC 58 | 1,900 | - | 100\% | 100\% | - | 4.1\% | - | - | 1.9 / 0 | $2 / 0$ |
| Entrance of Lenoir CC East of NC 59 | 1,300 | - | 100\% | 100\% | - | 11.1\% | - | - | 2.9 / 6.3 | $3 / 6$ |
| SR 2032 (Sanderson Way) East of SR 2003 (Industrial Way) * | 2,300 | - | 77\% | 70\% | - | 12.6\% | 12\% | - | $3 / 6$ | 3 / 8 |
| SR 1158 (Leslire R. Stroud Road) West of NC 11 | 300 | - | 60\% | 60\% | - | 8.9\% | 9\% | - | $7.1 / 0.9$ | 2 / 1 |

* Peak is during 5:15 AM-6:15 AM in morning and 2:15 PM-3:15 PM in afternoon due to adjacent industrial employment generator.


## APPENDIX C

# TRAFFIC ESTIMATES FOR SHALLOW BYPASS INTERCHANGE OPTIONS 

- Cl - Scenario S4-ABDE
- C2 - Scenario S4-ABCE
- C3-Scenario S3-ABE
- C4 - Scenario S2 - AE
- C5 - Scenario P3-ABD
- C6 - Scenario P2 - AD








[^0]:    * Turns only, throughs not counted

